#### **1285 Northern Boulevard**

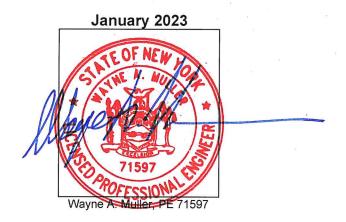
Construction of a BMW Automobile Dealership with No Service Department

## TRAFFIC IMPACT STUDY

Prepared in Accordance With Chapter 5 of the NYSDOT Highway Design Manual (HDM)



R&M Job Number 2021-196



**Note:** It is a violation of law for any person, unless they are acting under the direction of a licensed professional engineer, architect, landscape architect, or land surveyor, to alter an item in any way. If an item bearing the stamp of a licensed professional is altered, the altering engineer, architect, landscape architect, or land surveyor shall stamp the document and include the notation "altered by" followed by their signature, the date of such alteration, and a specific description of the alteration.

This report is based on the NYSDOT TIS Shell revised on 9/16/2014.

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#### INTRODUCTION

This report was conducted to determine the potential traffic and parking impacts associated with the redevelopment of the property located at 1285 Northern Boulevard (NYS Route 25A) in the Hamlet of Manhasset, Town of North Hempstead, Nassau County, New York. The subject site is located on the northeast corner of the intersection of Northern Boulevard at Norgate Road. The subject parcels are 77,327 sf (1.78 acres) in size and are represented on the Nassau County Tax Map (NCTM) as Section 3, Block 53, Lots 38, 53, 54, & 942. The property is situated within the Business A (B-A) Zoning district. The location of the project site is visually depicted in Figure 1 – Location Map contained in Appendix A.

The site currently contains a vacant funeral home. The project involves the demolition of the existing building and construction of a 2-story, 26,741 sf BMW automobile dealership . The automobile dealership **will not** provide a service department. Additionally. 5,445 sf of the dealership will be utilized as office space. A conditional use permit will be required for the parking, storage, and sale of automobiles. A visual depiction of the proposed project is provided on the Alignment Plan (SP-2) prepared by R&M Engineering dated April 29, 2022, and most recently revised October 26, 2022.

R&M conducted a detailed investigation of the potential traffic impacts of the proposed project on the surrounding roadway network. This report reviews the existing roadway and traffic conditions in the area, estimates the volume and pattern of traffic generated by the proposed project, and summarizes the results of the analyses performed. Additionally, the potential parking impacts on-site were examined.

#### 1.0 **PROJECT DESCRIPTION**

#### Existing Land Use

The 1.78 acre property is located on Northern Boulevard in Manhasset and is situated on northeast corner of the intersection of Northern Boulevard at Norgate Road. Presently, the site contains a vacant funeral home. The applicant is proposing to demolish the existing building and construct a 26,741 sf automobile dealership without a service department; 5,445 sf of the building area will be utilized as office space.

#### Zoning

The subject property situated within the Business A Zoning District. No change in zoning will be required as part of this application. A conditional use permit will be required for the parking, storage, and sale of automobiles.

#### <u>Access</u>

Currently, access to the site is provided via two (2) driveways:

- <u>East Site Driveway:</u> This driveway is located on the east side of the site on Northern Boulevard. This driveway is signalized and forms a 4-legged intersection with Northern Boulevard and Clapham Avenue. Upon construction of the project, this driveway will only provide ingress to the site and will prohibit egress.
- <u>West Site Driveway:</u> This driveway is located on the west side of the site on Northern Boulevard. This driveway is unsignalized and provides full access to and from the site. There are no planned changes to this access as part of the project.

#### <u>Parking</u>

As indicated on the Alignment Plan, the auto dealership will require one (1) parking space per 300 sf, and the proposed office will require one (1) parking space per 200 sf in excess of 1,000 sf. Therefore, the proposed project will require a total of 94 parking spaces. As depicted on the Alignment Plan, there will be 72 total parking spaces provided on-site. Therefore, a parking variance for 22 spaces will be required from the Town.

In addition to the 72 provided parking spaces, a proposed inventory storage area will be located on the eastern edge of the site (abutting the adjacent property). Additionally, there will be three (3) vehicle display areas in the front of the site fronting Northern Boulevard.

#### 2.0 STUDY METHODOLOGY

R&M's approach to conducting this investigation involved the following:

- 1. A detailed review of the existing land use, roadway characteristics, and traffic conditions in the vicinity of the project site was performed.
- Existing traffic volume data was collected by R&M Engineering on Thursday, June 16, 2022 during the Weekday AM Commuter (7:00 AM – 9:00 AM), Midday (11:00 AM – 3:00 PM), and PM Commuter (4:00 PM – 6:00 PM) peak periods and on Saturday, June 18, 2022 during the Midday peak period. The following intersections were studied:
  - 1. Northern Boulevard at Plandome Road
  - 2. Northern Boulevard at Clapham Avenue/East Site Driveway
  - 3. Northern Boulevard at Norgate Road

It should be noted that this traffic study was prepared and analysis performed during the ongoing COVID-19 Pandemic, which began in Mid-March 2020. As a result of the pandemic, traffic volumes

may be varied from what typical levels would be. Therefore, the observed traffic volumes were evaluated to determine if adjustments would be necessary to account for any decreased traffic activity as a result of the Pandemic. This will be discussed in greater detail later in this report.

- 3. The existing traffic volumes were analyzed to determine the intersections' "Existing" Levels of Service (LOS) at the study location. Capacity analyses to compute the intersection LOS were performed for both existing and future conditions. The future condition consisted of two scenarios: "No Build" and "Build."
- 4. The "No Build" condition analysis determined the future LOS at the study intersections assuming the project were not constructed. An ambient traffic growth factor of 0.5% per year, obtained from the **New York State Department of Transportation (NYSDOT)** and specific to the Town of North Hempstead, was applied to the collected 2022 traffic volumes for a period of 2 years to elevate the volumes to 2024 design levels. The "No Build" condition also included the traffic generated by other projects within the study area. As per conversations with the representatives of the Town of North Hempstead, there are two (2) other planned projects within the vicinity of the site that could potentially impact the study intersections.
- 5. The 2024 "Build" condition analysis considered the impact of the traffic generated by the construction of the proposed project. This site generated traffic was superimposed onto the 2024 "No Build" volumes at the study location to obtain the "Build" volumes.

#### COVID-19 Pandemic

This traffic study was prepared and analysis performed during the COVID-19 Pandemic of 2020, which began in Mid-March 2020. As a result of the Pandemic, traffic volumes may be varied from what is typical. In August of 2020, the **NYSDOT** released a memorandum titled "Traffic Data Collection Guidance during COVID-19 Pandemic," which provides guidelines on how to collect and interpret traffic data during the Pandemic. The traffic volume data collected by this firm as described above was evaluated to determine if adjustments were necessary to elevate the observed traffic volumes to non-pandemic conditions. The following describes our process:

 Historical traffic volume data in the form of Annual Average Daily Traffic (AADT) reports were obtained from the NYSDOT for the study roadways that traffic volume data was available for. The ADT Average Weekday (Axle Factored) volumes were utilized and elevated to year 2022 using a growth factor of 0.5% provided by the NYSDOT and specific to the Town of North Hempstead.

- 2. The observed volumes were compared to the elevated historical volumes, and a rate (historical data/observed data) was prepared for each approach. If the rate on an approach was less than or equal to 1.05, no adjustment was made on that approach. Based on our experience with adjusting traffic volumes during the Pandemic and our engineering judgement, it was determined that a rate of 1.05 or less represents traffic levels that are comparable to or greater than non-pandemic conditions (as roadway traffic volumes could fluctuate daily).
- 3. If traffic volume data was not available for Saturday, the average of the weekday rates on each approach was utilized to adjust the traffic volume data on that specific approach.
- 4. For Norgate Road, adjustments were determined to be unnecessary based on the number of approaches that did not require traffic volume adjustments. Additionally, Norgate Road is a local roadway, which would typically generate low levels of traffic activity.
- 5. Traffic volumes were balances between intersections to conserve traffic volumes between locations. Volumes were only increased, thus providing a conservative analysis.

The COVID-19 adjustments comparisons are included in Appendix C. The COVID-19 adjusted volumes were utilized to represent the "Existing" condition traffic volumes and are represented on Figure 2 located in Appendix A. The peak period traffic volumes at the study intersections are also tabulated in Appendix B.

#### 3.0 TRAVEL SPEEDS

#### Posted Speed Limit

The following are the posted speed limits for all the study roadways within the vicinity of the site:

- 1. Northern Boulevard: 35 mph
- 2. Plandome Road: 30 mph
- 3. Clapham Avenue: 30 mph
- 4. Norgate Road: 25 mph

#### Actual Operating Speed

The off-peak 85th percentile speed based on **NYSDOT** Highway Design Manual Chapter 5 Section 5.2 determines the existing operational travel speeds on the segments of interest. According to the **NYSDOT** Traffic Data Viewer, available on the **NYSDOT** website, the 85<sup>th</sup> percentile speed for Plandome Road is 25 mph in both directions of travel, and for Clapham Avenue the 85<sup>th</sup> percentile speed is 23 mph and 22 mph in the northbound and southbound directions or travel, respectively. The **NYSDOT** did not have any 85<sup>th</sup> percentile speed data for Northern Boulevard.

#### 4.0 CRASH ANALYSIS

As part of this study, a crash analysis of the roadway network surrounding the site was performed. Accident summaries for a 3-year period from January 1, 2017 to December 31, 2019 were obtained from the **NYSDOT**. Accident data was gathered for the three study intersections listed in the Study Methodology section of this report. Based on the information received, 102 accidents were reported at the locations requested; 7 (7%) resulted in injury, no fatal accidents were recorded. The **NYSDOT** Traffic Data Viewer was utilized to estimate AADT volumes at these intersections. Inspection of the data reveals the following:

#### Northern Boulevard at Plandome Road

A total of 70 accidents occurred at this location during the three year period in which accident data was gathered for. This equates to a rate of 23.33 accidents per year (acc/year). Of the 70 accidents reported, 5 accidents resulted in injury; no fatal accidents were recorded. The most frequent accident types were rear end collisions (38 incidents – 54%) and overtaking collisions (16 incidents – 23%). The approximate AADT experienced at this intersection is 37,628 vehicles, equating to 41.20 million entering vehicles (mev) over the course of 3 years. As such, there is an accident rate of 1.70 acc/mev per year at this intersection. The statewide average for an intersection such as this one is 0.17 acc/mev per year. The accident rate at this intersection is above the statewide average.

#### Northern Boulevard at Clapham Avenue/East Site Driveway

A total of 11 accidents occurred at this location during the three year period in which accident data was gathered for. This equates to a rate of 3.67 acc/year. Of the 11 accidents reported, 1 accidents resulted in injury; no fatal accidents were recorded. The most frequent accident type was rear end collisions (6 incidents – 55%). The approximate AADT experienced at this intersection is 29,990 vehicles, equating to 32.84 mev over the course of 3 years. As such, there is an accident rate of 0.33 acc/mev per year at this intersection. The statewide average for an intersection such as this one is 0.26 acc/mev per year. The accident rate at this intersection is above the statewide average.

#### Northern Boulevard at Clapham Avenue

A total of 21 accidents occurred at this location during the three year period in which accident data was gathered for. This equates to a rate of 7 acc/year. Of the 21 accidents reported, 1 accidents resulted in injury; no fatal accidents were recorded. The most frequent accident types were rear end collisions (7 incidents – 33%) and right angle collisions (6 incidents – 29%). The approximate AADT experienced at this intersection is 29,853 vehicles, equating to 32.39 mev over the course of 3 years. As such, there is an accident rate of 0.65 acc/mev per year at this intersection. The statewide average for an intersection such as this one is 0.07 acc/mev per year. The accident rate at this intersection is above the statewide average.

A summary of the accidents that occurred within the study area is included on Table 7 of Appendix A. Based on this information, the average accident rate at the intersections above were above the statewide average. It should be noted that our experience throughout Long Island is that accident rates on Long Island are typically higher than the statewide average.

#### 5.0 CAPACITY ANALYSIS

The results of the capacity analyses were used to determine the potential impact of the proposed development. A comparison of the "Build" condition to the "No Build" condition determines if the proposed development has the potential to produce an impact on traffic conditions on the roadway network in the vicinity of the site.

#### Capacity Analysis Overview

The capacity analyses performed in this report are consistent with the methodologies presented in the most recent version of the **Highway Capacity Manual (HCM 6)** published by the Transportation Research Board. The **Synchro 11** software developed by **Trafficware** was used to perform the analyses.

The **HCM 6** quantifies the quality of traffic flow in terms of Levels of Service (LOS). There are six levels of service; LOS A – D indicate the intersection is operating under capacity with low levels of delay, LOS E indicates the intersection is operating at capacity, and LOS F indicates the intersection is operating over capacity with high levels of delay. These represent a qualitative measure of operational conditions within a traffic stream and the perception of conditions by motorists and/or passengers.

LOS and capacity for signalized intersections are calculated for each lane group (a lane group may consist of one or more movements), each intersection approach, and the intersection as a whole. An intersection's LOS is a weighted average of the individual approaches and may not be considered a valid measure of the quality or acceptability of an intersection design since it can conceal poor operating conditions on individual approaches. The LOS at unsignalized intersections is only calculated for minor movements since the through movement on the major street is not affected by intersection traffic control.

The LOS for freeway facilities is a measurement of density expressed as the number of passenger car equivalents/lane/mile. The corresponding LOS represents the congestion of the roadway:

LOS for Signaliz	zed Intersect	ions	LOS for N	on-signa	alized Intersect	tions
	LOS by Vo	olume-to-Capacity			LOS by Vol	ume-to-Capacity
	Ratio (v/c)				Ratio (v/c) <sup>a, b</sup>	
Control Delay	v/c ≤1.0	v/c >1.0	Control	Delay	v/c ≤1.0	v/c >1.0
(s/veh)			(s/veh)			
≤10	А	F	≤10		А	F
>10-20	В	F	>10-15		В	F
>20-35	С	F	>15-25		С	F
>35-55	D	F	>25-35		D	F
>55-80	E	F	>35-50		E	F
>80	F	F	>50		F	F
HCM 6 <sup>th</sup> Edition, E	xhibit 19-8, p.	19-16	2-way stop	control -	HCM 6th Edition	, Exhibit 20-2, p. 20-0

NOTE: <sup>a</sup>, <sup>b</sup> For approaches and intersection-wide assessment, LOS is defined solely by control delay.

<sup>a</sup> All way stop control - HCM 6th Edition, Exhibit 21-8, p. 21-9

<sup>b</sup> Roundabout control - HCM 6th Edition, Exhibit 22-8, p. 22-9

#### 5.1 Growth Rates

An ambient traffic growth rate factor of 0.5% per year, supplied by the **NYSDOT** and specific to the Town of North Hempstead, was applied to the collected 2022 traffic volumes for a period of 2 years for expansion to year 2024 design levels.

#### 5.2 Existing Traffic Counts and Volume Data as AADT/DHV

#### Roadway Network

Current roadway conditions in the study area are summarized below:

- 1. <u>Northern Boulevard:</u> This is an east/west principal arterial "other" roadway under the jurisdiction of the **NYSDOT** and is designated at NYS Route 25A. Northern Boulevard provides two travel lanes in each direction and dedicated turn lanes where appropriate. Northern Boulevard provides a Two-Way-Left-Turn-Lane (TWLTL) within the vicinity of the site. Access to the site will be provided via Northern Boulevard at two driveways. According to the **NYSDOT** Traffic Data Viewer, the AADT within the vicinity of the site is 29,583 vehicles based on 2019 actual volumes. The posted speed limit within the vicinity of the site is 35 mph.
- 2. <u>Plandome Road:</u> This is a north/south minor arterial roadway under the jurisdiction of **Town of North Hempstead** within the vicinity of the site. Plandome Road provides one travel lane in both directions. At its intersection with Northern Boulevard, Plandome Road provides two dedicated left turn lanes and a dedicated right turn lane. According to the **NYSDOT** Traffic Data Viewer, the AADT

within the vicinity of the site is approximately 15,103 vehicles based on the 2019 estimated volumes. The posted speed limit within the vicinity of the site is 30 mph.

- 3. <u>Clapham Avenue:</u> This is a north/south major collector roadway under the jurisdiction of the **Town** of North Hempstead. Clapham Avenue provides a one travel lane in both directions. At its intersection with Northern Boulevard, Clapham Avenue provides one left turn lane and one right turn lane. There is a heavy vehicle restriction on Clapham Avenue prohibiting vehicles in excess of 4 tons to travel on the roadway. According to the NYSDOT Traffic Data Viewer, the AADT within the vicinity of the site is approximately 801 vehicles based on 2019 actual volumes. The implied speed limit is 30 mph.
- 4. <u>Norgate Road:</u> This is a north/south local roadway under the jurisdiction of the **Town of North Hempstead**. Norgate Road provides one travel lane in both directions. At its intersection with Northern Boulevard, Norgate Road provides one approach for all turning movements. The **NYSDOT** Traffic Data Viewer does not provide traffic volume data for this roadway. The posted speed limit is 25 mph.

#### Study Intersections

The following are the traffic control conditions at the study intersections:

- 1. <u>Northern Boulevard at Plandome Road:</u> Signalized intersection controlled by a multi-phase traffic signal with protected eastbound left turns and southbound left turns. Southbound right turns are protected with the eastbound left turns and are not called during the southbound left turn phase.
- 2. <u>Northern Boulevard at Clapham Avenue/East Site Driveway:</u> Signalized intersection controlled by a multi-phase traffic signal with protected and permitted westbound left turns. The southbound approach displays a flashing red light when the northbound/southbound phase is not called. Upon construction of the project, southbound traffic will not be permitted at this intersection.
- 3. <u>Northern Boulevard at Norgate Road:</u> Unsignalized intersection with stop control implemented on the southbound approach.
- 4. <u>Northern Boulevard at West Site Driveway:</u> Unsignalized intersection with stop control implemented on the southbound (driveway) approach.

#### Traffic Volume Data

Traffic volume turning movement counts were collected as described in the Study Methodology section of this report and were adjusted as necessary for any discrepancies in traffic levels as a result of the COVID-19 Pandemic. Figure 2 located in Appendix A contains a visual depiction of the existing traffic volumes at these locations. These volumes are also tabulated in Appendix B.

#### 5.3 **Projected Trip Generation**

The proposed project involves the demolition of the existing building and the construction of an automobile dealership. A trip generation analysis was prepared for the proposed project using industry standard data presented in the **Institute of Transportation Engineers (ITE) Trip Generation Manual, 11<sup>th</sup> Edition.** Statistics under Land Use Code (LUC) 840 relating to Automobile Sales (New) were employed. The following is a summary of the estimated peak hour trips to be generated by the project:

- AM Commuter Peak Hour: 50 trips (36 entering, 14 exiting)
- Midday Peak Hour: 76 trips (35 entering, 41 exiting)
- PM Commuter Peak Hour: 65 trips (26 entering, 39 exiting)
- Saturday Peak Hour: 107 trips (53 entering, 54 exiting)

This information has been summarized and is included in Table 1 located in Appendix A. It should be noted that the proposed dealership <u>will not</u> offer a service department and will primarily function as a sales center for automobiles. According to the ITE, a new automobile sales dealership commonly provides automobile service, parts sales, and used car sales. While the ITE does not provide data on which sites surveyed offered a service center, it should be assumed that some of their data reflect the presence of a service center, and therefore, the number of trips estimated above are likely greater than what would actually be generated by the project.

#### 5.4 Access & Parking

#### <u>Access</u>

Currently, access to the site is provided via two (2) driveways:

 <u>East Site Driveway:</u> This driveway is located on the east side of the site on Northern Boulevard. This driveway is signalized and forms a 4-legged intersection with Northern Boulevard and Clapham Avenue. Upon construction of the project, this driveway will only provide ingress to the site and will prohibit egress.  <u>West Site Driveway:</u> This driveway is located on the west side of the site on Northern Boulevard. This driveway is unsignalized and provides full access to and from the site. There are no planned changes to this access as part of the project.

#### Parking

As indicated on the Alignment Plan, the auto dealership will require one (1) parking space per 300 sf, and the proposed office will require one (1) parking space per 200 sf in excess of 1,000 sf. Therefore, the proposed project will require a total of 94 parking spaces. As depicted on the Alignment Plan, there will be 72 total parking spaces provided on-site. Therefore, a parking variance for 22 spaces will be required from the Town.

In addition to the 72 provided parking spaces, a proposed inventory storage area will be located on the eastern edge of the site (abutting the adjacent property). Additionally, there will be three (3) vehicle display areas in the front of the site fronting Northern Boulevard.

In order to estimate the parking activity to be generated by the project, a parking generation estimate was prepared using industry standard data presented in the **ITE Parking Generation Manual**, 5<sup>th</sup> Edition. Statistics under LUC 840 relating to Automobile Sales (New) were employed. The following is a summary of the anticipated peak parking demand for the proposed project:

Weekday: 2.29 vehicles parked per 1,000 sf x 26,741 sf = 62 vehicles parked (86% occupied)

This information is included in Table 2 of Appendix A. Based on the information above, the peak parking demand is expected to occur during the Weekday with 62 vehicles parked. This would leave 10 vacant parking spaces on-site. Therefore, we believe that adequate parking is provided on-site to accommodate the parking activity to be generated by the proposed dealership.

We would like to note that the dealership <u>will not</u> offer a service center. According to the ITE, a new automobile sales dealership commonly provides automobile service, parts sales, and used car sales. While the ITE does not provide data on which sites surveyed offered a service center, it should be assumed that some of their data reflect the presence of a service center, and therefore, the number of vehicles parked estimated above could be greater than what would actually be experienced by the project.

#### 5.5 Traffic Control Device Data

In the "Existing" condition, the signalized intersections on Northern Boulevard are controlled by multiphased controllers. All timings and phasing utilized to represent the existing condition were obtained via the **NYSDOT** and were field verified by the representatives of R&M Engineering to assure that the simulated results match the current operation. The two unsignalized intersections are controlled by two-way stop control on the southbound approaches. Any proposed modifications will be discussed later in this report as applicable.

#### 5.6 Capacity Analysis for No Build Condition

The 2024 "No Build" condition analysis determined the future LOS at the study intersections assuming the project were not constructed. In order to determine the "No Build" traffic volumes generated within the study area, a 0.5% growth factor, obtained from the **NYSDOT** and specific to the Town of North Hempstead, was applied to the "Existing" condition traffic volumes for a period of 2 years to project traffic volumes to 2024 design conditions.

The "No Build" condition also considered traffic to be generated by other planned projects in the study area. Based on our communication with representatives of the Town of North Hempstead, there are two (2) other planned projects within the study area that would generate a significant level of traffic and could potentially impact the study intersections. The following is a description of each planned project:

#### Lord & Taylor Building

This project is located at 1440 Northern Boulevard on the southeast corner of the intersection of Northern Boulevard and Shelter Rock Road. The site contains a 162,750 sf vacant building that was previously occupied by Lord & Taylor (retail). According to the Town, it is likely that this building will be converted to a Medical Office, although no application has been submitted as of this writing. However, the traffic to be generated by the potential conversion of this building has been estimated and accounted for in this analysis. A trip generation estimate for this project was prepared using industry standard data presented in the **ITE Trip Generation Manual, 11<sup>th</sup> Edition** under LUC 720 relating to Medical Office (Stand-Alone). These trips were distributed into the study intersections as appropriate. The following is a summary of the peak hour trips to be generated by the project:

- AM Commuter Peak Hour: 505 trips (399 entering, 106 exiting)
- Midday Peak Hour: 586 trips (325 entering, 261 exiting)
- PM Commuter Peak Hour: 640 trips (192 entering, 448 exiting)
- Saturday Peak Hour: 492 trips (280 entering, 212 exiting)

#### 1575 Northern Boulevard

This project is located at 1575 Northern Boulevard on the north side of Northern Boulevard. The site contains two buildings, and the project involves changes to both buildings. Building A is a vacant 6,310 sf building (previously retail). Building A will be partially demolished (1,142 sf) and reoccupied with a retail use. Building B is a partially vacant 14,961 sf building, occupied with Investors Bank (2,249 sf), Mayweather

Boxing + Fitness (2,249 sf), and Mystique Boutique NYC (5,881 sf). Building B will be partially demolished (5,881 sf) so that Building B becomes two separate structures (Building B and Building C). Investors Bank and Mayweather Boxing + Fitness will remain tenants and make up the entirety of Building B. Building C will consist of a 2,858 sf Fast Casual Restaurant and a 1,670 sf Medical Office. Trip generation estimates for this project were obtained directly from the Traffic and Parking Assessment prepared by Creighton Manning dated July 20, 2022. These trips were distributed into the study intersections based on the distributions presented within the Traffic and Parking Assessment and a review of the surrounding roadway network. The following is a summary of the peak hour trips to be generated by the project:

- AM Commuter Peak Hour: 24 trips (20 entering, 4 exiting)
- Midday Peak Hour: 91 trips (47 entering, 44 exiting)
- PM Commuter Peak Hour: 113 trips (62 entering, 51 exiting)
- Saturday Peak Hour: 137 trips (75 entering, 62 exiting)

The traffic activity generated by the other planned projects was included in the "No Build" condition. A summary of the trip generation estimates of the above projects is included in Table 3 of Appendix A. Appendix B contains a tabular summary of the vehicle trips that will impact the study intersections and the "No Build" traffic volumes. Figure 3, located in Appendix A, contains a visual depiction of the "No Build" traffic volumes.

The results of the "No Build" condition analysis for the signalized and unsignalized intersections are contained in Tables 4 - 6 located in Appendix A. The results indicate that, from an overall perspective, the study intersections operate adequately during all time periods studied.

#### 5.7 Capacity Analysis for Proposed Build Condition

The 2024 "Build" condition analysis determined the future LOS at the study intersections assuming the project were constructed. The proposed project traffic volumes, determined by the percent distribution of the site-generated traffic, were added to the "No Build" traffic volumes to obtain the "Build" traffic volumes and were used to calculate the LOS.

In order to determine the distribution of the traffic generated by the site, a careful review of the existing travel patterns and the trip generators/receptors in the vicinity of the site were examined.

#### Traffic Distribution and Assignment

1. The next step of the investigation consisted of an analysis of the geographical distribution of the traffic to and from the site.

- 2. In order to properly assess the traffic impact of the project, it was necessary to determine which roadway(s) will most probably receive the newly generated traffic during the peak periods studied. Arrival/departure patterns for the site were developed using the existing traffic volumes, traffic patterns, and the geometry of the surrounding roadway network. Using this information and our engineering judgement, it was determined that a different distribution pattern for each time period was necessary in order to ensure the most accurate analysis within the study area.
- 3. Refer to Appendix B for a tabular summary of the trip distribution/assignment, increase in volumes from the "No Build" to the "Build" condition and for the "Build" volumes. The percent distribution, shown in Figure 4, was applied to the traffic generated by the proposed project. The site generated traffic, depicted in Figure 5, was superimposed onto the "No Build" volumes to obtain the :Build" volumes, depicted in Figure 6. All figures are contained within Appendix A.

Based on the "Build" traffic volumes, the proposed "Build" LOS calculations for each intersection within the study area were computed using Synchro 11. The capacity and LOS of an intersection have previously been defined in Section 5.0 of this report. The worksheet reports from the capacity analysis are contained in Appendix D, and the results of the capacity analyses are summarized in Tables 4 - 6 contained in Appendix A.

After reviewing the information contained in Tables 4 - 6, an investigation was undertaken to determine whether any measures would be required to improve the operation of the roadway network after the construction of the proposed project. All delay and LOS values are in comparison to the "No Build" condition. The results of this analysis are as follows:

#### Northern Boulevard at Plandome Road:

Upon the introduction of the site-generated traffic, this signalized intersection will experience a minor increase in delay and no degradation in LOS for all time periods studied. Based on a detailed review of the analyses, no mitigation will be required at this location:

- AM Commuter Peak Hour: 0.3 second increase in delay; Remains at LOS D
- Midday Peak Hour: 0.4 second increase in delay; Remains at LOS D
- PM Commuter Peak Hour: 0.6 second increase in delay; Remains at LOS D
- Saturday Peak Hour: 0.7 second increase in delay; Remains at LOS D

#### Northern Boulevard at Clapham Avenue/East Site Driveway:

Upon the introduction of the site-generated traffic, this signalized intersection will experience a minor increase in delay and no degradation in LOS for all time periods studied. Based on a detailed review of the analyses, no mitigation will be required at this location:

- AM Commuter Peak Hour: No change in delay; Remains at LOS A
- Midday Peak Hour: 0.1 second increase in delay; Remains at LOS A
- PM Commuter Peak Hour: 0.1 second increase in delay; Remains at LOS A
- Saturday Peak Hour: 0.1 second increase in delay; Remains at LOS A

#### Northern Boulevard at Norgate Road:

Upon the introduction of the site-generated traffic, this unsignalized intersection will not experience any significant increases in delay nor degradation in LOS during all time periods studied. Based on a detailed review of the analyses, no mitigation will be required at this location.

#### Northern Boulevard at West Site Driveway:

Upon the introduction of the site-generated traffic, all approaches will operate at acceptable levels of service. Based on a detailed review of the analyses, no mitigation will be required at this location.

#### 5.8 Mitigation Measures

Based on the analyses presented herein, it is the opinion of R&M Engineering that the traffic generated by the proposed action will not have a significant impact on the operation of the surrounding roadway network when compared to the "No Build" condition. Therefore, we believe that mitigation is not required.

#### 6.0 SUMMARY OF TRAFFIC IMPACTS

In accordance with the analysis above, the following conclusions have been made regarding the impacts upon the roadway network as a result of the traffic generated by the proposed development:

- 1. With the addition of the project traffic to the "No Build" condition, the analyses show that during the peak periods analyzed there will be a minor change to the time delay and no degradation in LOS at each studied intersection. The traffic generated by the proposed development will maintain an acceptable LOS in the future condition in accordance with Section 5.0 of this report. No mitigation will be required at any of the studied intersections.
- 2. Based on the parking requirements outlined by the Town Code, the site will provide 22 less spaces than what is required by the Town (94 required vs. 72 provided) and will require a variance in parking. The parking analysis contained herein indicates that adequate parking will be provided onsite to accommodate the anticipated parking activity.
- 3. The dealership proposed will not provide a service department. While the **ITE** does not say which dealerships studied contained a service department, it does indicate that automobile sales as analyzed in this report commonly contain a service department; as such it can be assumed that

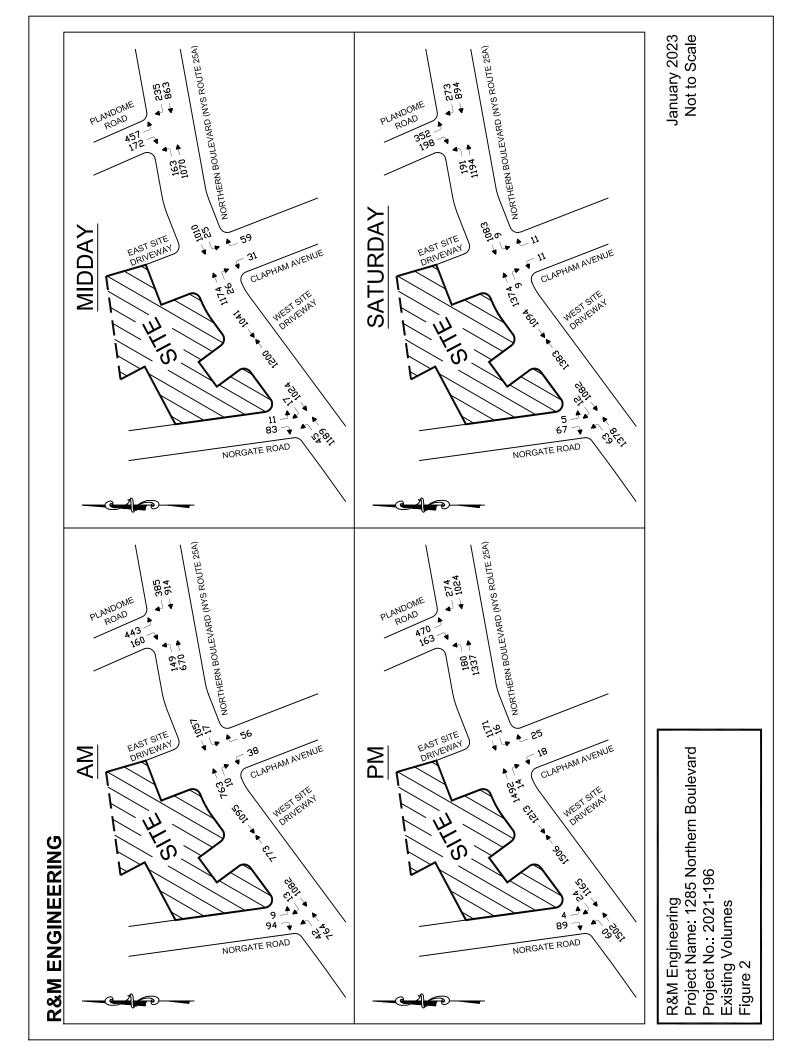
some of the industry standard data show the effect of a service department on a car dealership. Therefore, we believe that the estimated parking and traffic activity at the site could be less than what is presented in this report, as a service department would likely generate additional traffic at the site outside of car sales and other services.

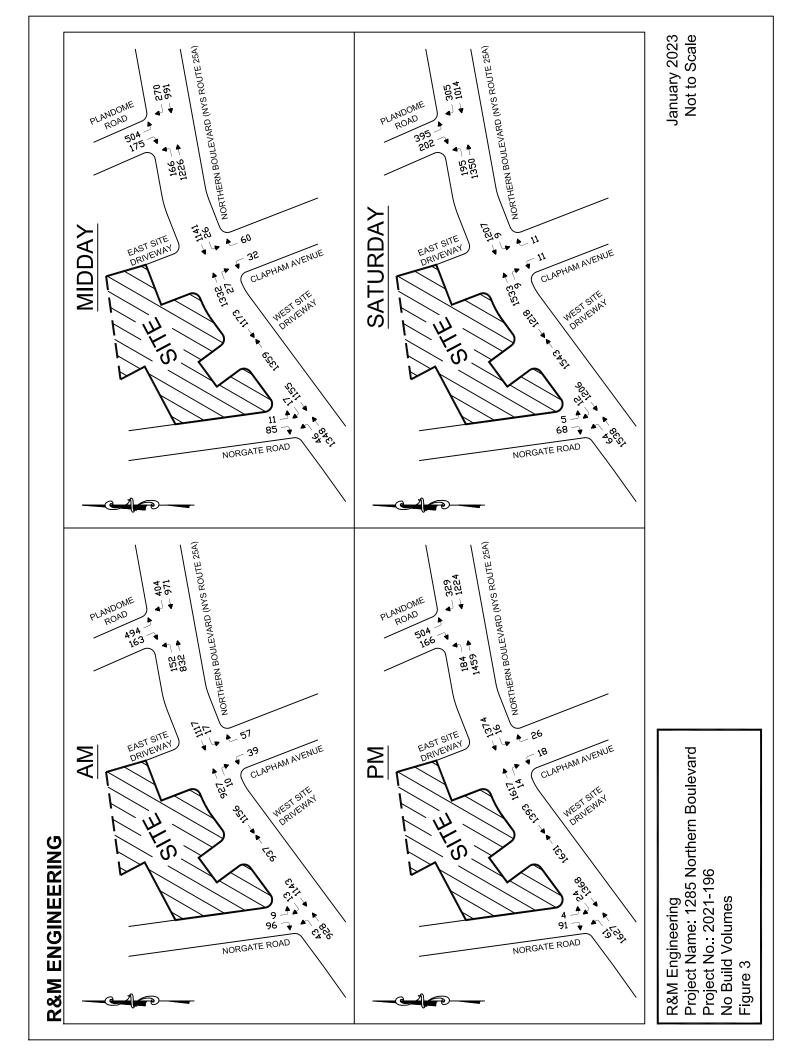
4. It should be noted that there are dealerships within close proximity to the site, including a Honda dealership directly across from the site. Therefore, we believe that the proposed project is not out of character with other uses in the area.

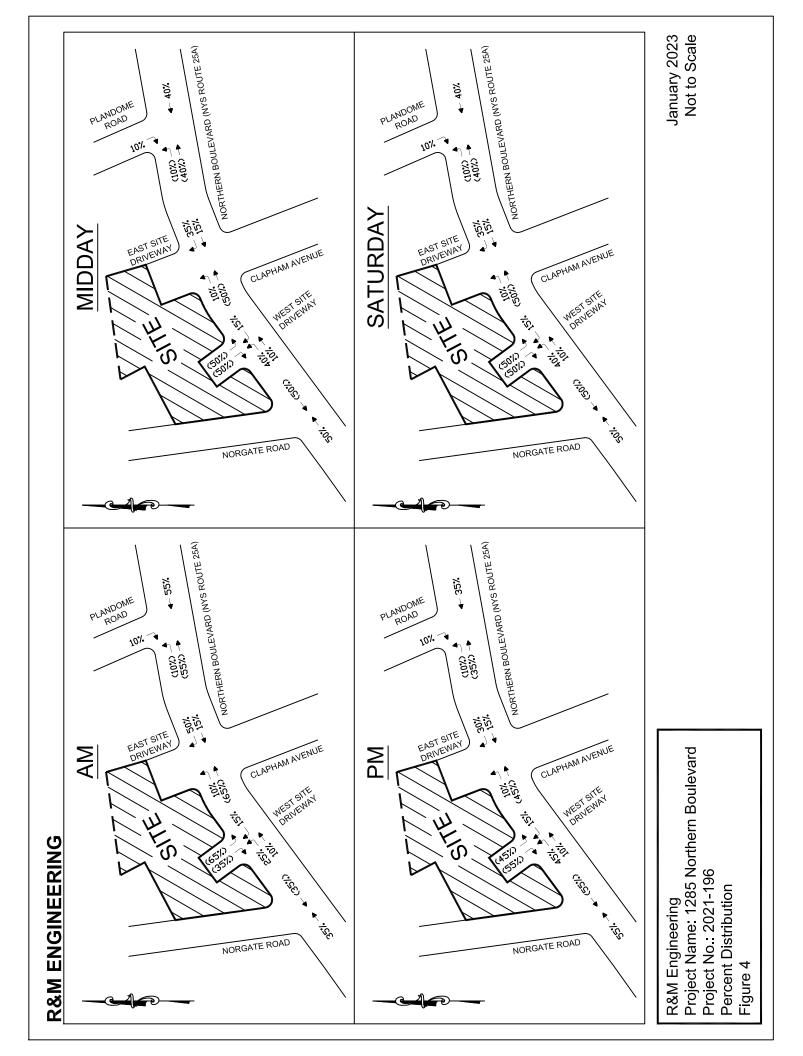
### APPENDIX A

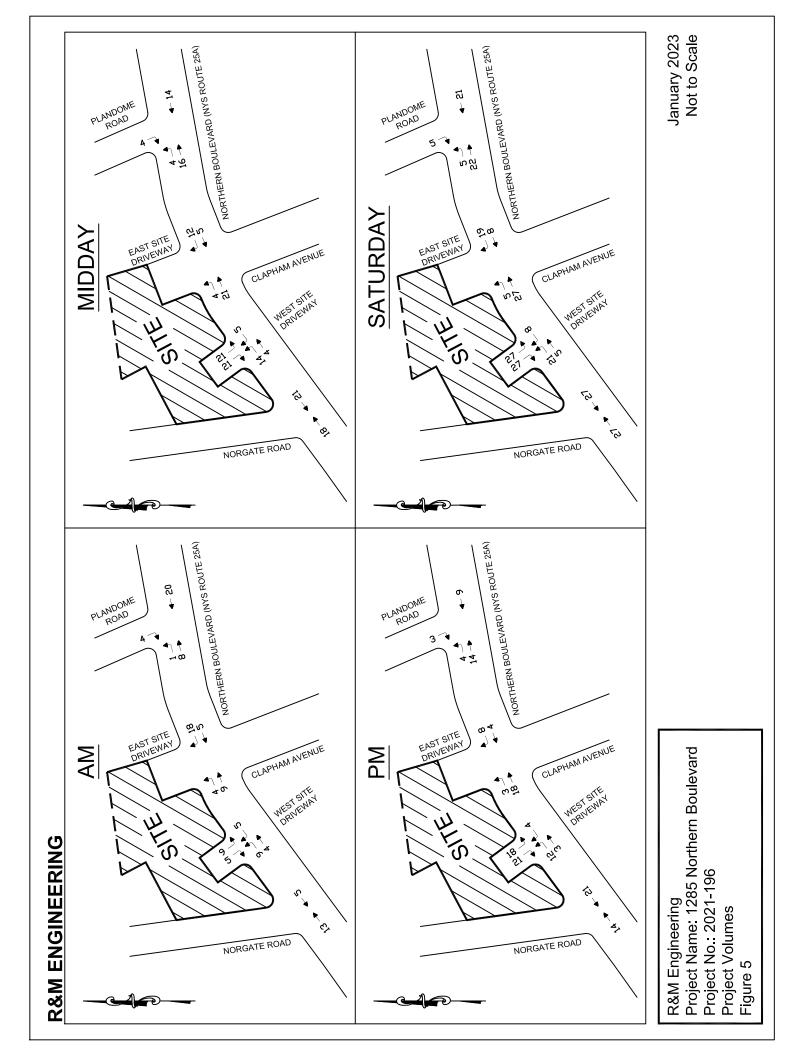
#### FIGURES AND TABLES

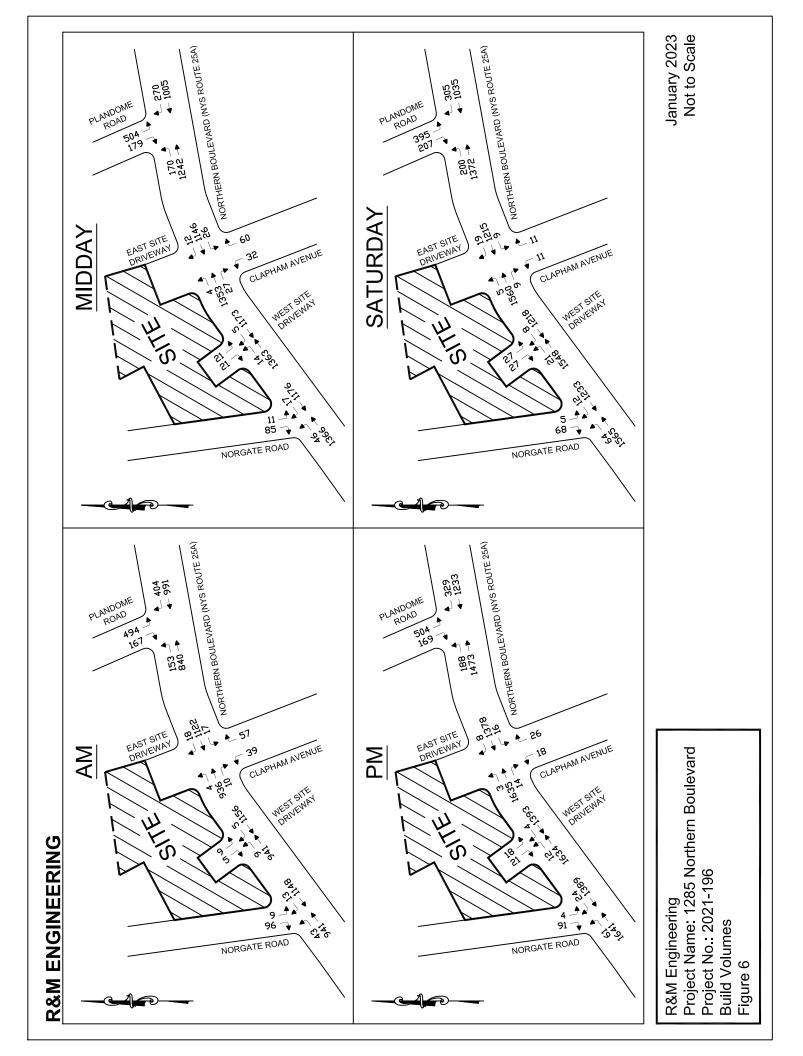
## R&M Engineering Project Name: 1285 Northern Boulevard Project No.: 2021-196 Figure 1 - Location Map January 2023 Not To Scale NORTHERN BOULEVARD (NYS ROUTE 25A) AT PLANDOME ROAD NORTHERN BOULEVARD AT CLAPHAM AVENUE/EAST SITE DRIVEWAY NORTHERN BOULEVARD AT NORGATE ROAD NORTHERN BOULEVARD AT WEST SITE DRIVEWAY STUDY INTERSECTIONS: **R&M ENGINEERING**











#### TABLE 1

1285 NORTHERN BOULEVARD TRIP GENERATION SUMMARY: PROPOSED PROJECT R&M JOB No. 2021-196 JANUARY 2023

		PROPOSED	
		AUTOMOBILE SALES (NEW)	
		26,741 SF	
PEAK HOUR		(ITE RATES)	
ITE LAND USE CC	DE:	840*	
AM COMMUTER	ENTER:	36 tph*	
PEAK HOUR	EXIT:	<u>14</u> <u>tph*</u>	
	TOTAL:	50 tph*	
MIDDAY PEAK HOUR	ENTER: <u>EXIT:</u> TOTAL:	35 tph* <u>41</u> <u>tph*</u> 76 tph*	
PM COMMUTER	ENTER:	26 tph*	
PEAK HOUR	EXIT:	<u>39</u> tph*	
	TOTAL:	65 tph*	
SATURDAY PEAK HOUR	ENTER: <u>EXIT:</u> TOTAL:	53 tph* <u>54</u> <u>tph*</u> 107 tph*	

\* Based on ITE Trip Generation Manual, 11th Edition

tph - trips per hour

**TABLE 2** 

1285 NORTHERN BOULEVARD PARKING GENERATION SUMMARY R&M JOB No. 2021-196 JANUARY 2023

	PROPOSED			
	AUTOMOBILE SALES (NEW)			VACANT
	26,741 SF			SPACES
PEAK HOUR	(ITE RATES)			
911 1	*UVØ			
	0+0			
FRIDAY	62 maximum parked*	72	86%	10
PEAK PERIOD	2.29 vehicles parked per 1,000 SF*			
	O	-		

\* Based on ITE Parking Generation Manual, 5th Edition

**TABLE 3** 

1285 NORTHERN BOULEVARD TRIP GENERATION SUMMARY: OTHER PLANNED PROJECTS R&M JOB No. 2021-196 JANUARY 2023

		PROPOSED MEDICAL OFFICE	PROPOSED MIXED-LISE DEVELOPMENT
		INEDICAL UFFICE 162,750 SF	MIAEU-USE DEVELOFMENT 14,194 SF
PEAK HOUR	R	(ITE RATES)	(ITE RATES)
ITE LAND USE CODE:	CODE:	720*	* *
AM COMMUTER PEAK HOUR	ENTER: EXIT:	399 tph* 106 tph*	20 tph** 4 tph**
	TOTAL:	505 tph*	24 tph**
MIDDAY	ENTER:	325 tph*	47 tph**
PEAK HOUR	<u>EXIT:</u>	<u>261 tph*</u>	<u>44</u> tph**
	TOTAL:	586 tph*	91 tph**
PM COMMUTER	ENTER:	192 tph*	62 tph**
PEAK HOUR	EXIT:	<u>448</u> tph*	51 tph**
	TOTAL:	640 tph*	113 tph**
SATURDAY	ENTER:	280 tph*	75 tph**
PEAK HOUR	<u>EXIT:</u>	<u>212 tph*</u>	<u>62</u> tph**
	TOTAL:	492 tph*	137 tph**
* Based on ITE Trip Generation Manual, 11th Edition	ation Manual, 1	1th Edition	

\*\* Obtained from the Traffic and Parking Study prepared by Creighton Manning, dated July 20, 2022 tph - trips per hour

1285 NORTHERN BOULEVARD LEVEL OF SERVICE SUMMARY SIGNALIZED INTERSECTION R&M JOB No. 2021-196 JANUARY 2023

		AM COM	AM COMMUTER PEAK HOUR	our	MIDD	MIDDAY PEAK HOUR	r	PM COM	PM COMMUTER PEAK HOUR	HOUR	SATUF	SATURDAY PEAK HOUR	R
INTERSECTION	CONDITION	MVMNT	I UIAL DELAY (SEC/VEH)	SOJ	MVMNT	I UIAL DELAY (SEC/VEH)	LOS	MVMNT	I U I AL DELAY (SEC/VEH)	SOJ	MVMNT	I UIAL DELAY (SEC/VEH)	SOJ
		EBL	105.7	ш	EBL	0.06	ш	EBL	123.6	ш	EBL	89.6	ш
		EBT	13.8	В	EBT	18.4	В	EBT	20.0	В	EBT	20.5	U
	EXISTING	WBTR	25.3	υ	WBTR	24.8	U	WBTR	25.1	υ	WBTR	24.2	U
		SBL	75.0	ш	SBL	84.1	ш	SBL	76.1	ш	SBL	69.1	ш
		SBR	94.6	ш	SBR	87.3	ш	SBR	96.4	ш	SBR	89.3	ш
		OVERALL	39.1	D	OVERALL	38.9	D	OVERALL	38.6	۵	OVERALL	36.0	D
		EBL	104.2	ш	EBL	92.1	ш	EBL	126.3	ш	EBL	91.5	ш
NORTHERN BOULEVARD		EBT	15.2	В	EBT	20.7	U	EBT	22.4	ပ	EBT	23.5	U
(NYS ROUTE 25A)		WBTR	27.5	U	WBTR	28.3	U	WBTR	31.2	U	WBTR	27.7	U
АТ		SBL	78.6	ш	SBL	104.6	ш	SBL	79.1	ш	SBL	71.8	ш
PLANDOME ROAD		SBR	97.4	ш	SBR	88.5	ш	SBR	97.5	ш	SBR	93.0	ш
		OVERALL	40.1	D	OVERALL	43.3	D	OVERALL	41.5	۵	OVERALL	38.5	۵
		EBL	103.9	ш	EBL	93.8	ш	EBL	131.7	ш	EBL	93.5	ш
		EBT	15.4	в	EBT	21.0	ပ	EBT	22.8	ပ	EBT	24.2	U
		WBTR	28.1	υ	WBTR	28.7	ပ	WBTR	31.6	U	WBTR	28.3	U
		SBL	78.6	ш	SBL	104.6	ш	SBL	79.1	ш	SBL	71.8	ш
		SBR	99.2	ш	SBR	0.06	ш	SBR	98.8	ш	SBR	94.1	ш
		OVERALL	40.4	D	OVERALL	43.7	D	OVERALL	42.1	۵	OVERALL	39.2	
LOS - Level of Service													]

LOS - Level of Service MVMNT - Movement

1285 NORTHERN BOULEVARD LEVEL OF SERVICE SUMMARY SIGNALIZED INTERSECTION R&M JOB No. 2021-196 JANUARY 2023

		AM COM	AM COMMUTER PEAK HOUR	IOUR	DOIM	MIDDAY PEAK HOUR	£	PM COM	PM COMMUTER PEAK HOUR	IOUR	SATUI	SATURDAY PEAK HOUR	R
INTERSECTION	CONDITION	MVMNT	TOTAL DELAY (SEC/VEH)	ros	MVMNT	TOTAL DELAY (SEC/VEH)	ros	MVMNT	TOTAL DELAY (SEC/VEH)	ros	MVMNT	TOTAL DELAY (SEC/VEH)	ros
	EXISTING	EBL EBTR WBL WBTR NBLT	0.0 5.0 0.4 1.0 76.4	<b>сссс</b> ш	EBL EBTR WBL WBTR NBLT	0.0 6.7 0.3 0.5 69.5	<b>ЧЧЧЧ</b>	EBL EBTR WBL WBTR NBLT	0.0 6.0 0.8 1.4 72.5	<b>ЧЧЧЧ</b>	EBL EBTR WBL WBTR NBLT	0.0 4.3 0.1 0.4 65.5	<b>ح ح ح ح</b> س
		NBR OVERALL	14.2 4.5	A B	NBR OVERALL	14.1 4.9	A B	NBR OVERALL	2.7 4.4	A A	NBR OVERALL	0.8 2.9	۲ ۲
NORTHERN BOULEVARD (NYS ROUTE 25A) AT CLAPHAM AVENUE/ EAST SITE DRIVEWAY	BUILD BUILD	EBL EBTR WBL WBLT WBLT NBLT NBL WBL WBL WBL WBL WBL NBL NBL NBL OVERALL	0.0 5.5 1.0 76.4 4.6 7.5 7.5 7.5 7.6 4.6 7.6 4.6 4.6		EBL EBTR WBL WBLT WBLT NBLT NBR OVERALL EBTR WBL WBL WBL NBLT NBLT NBLT	0.0 7.5 0.6 69.6 69.6 7.6 7.6 7.6 7.6 7.6 7.6 7.6 7.6 7.5 7.3 5.3 7.6 5.3 5.3 5.3 5.3	444404 444404	EBL EBTR WBL WBLT WBLT NBLT NBL WBL WBL WBL WBL WBL NBLT NBL NBL OVERALL	0.0 6.6 1.5 72.5 6.7 72.5 72.5 72.5 72.5 72.5	< < < < u < < < < < < < < < < < < < < <	EBL EBTR WBL WBLT WBLT NBLT NBR OVERALL EBTR WBL WBL WBL NBL NBL NBL NBL OVERALL	0.0 0.2 0.5 0.5 0.8 0.8 0.5 0.2 0.5 0.5 0.5 0.3 3.3 3.3	< < < < u < < < < < < < < < < < < < < <

LOS - Level of Service MVMNT - Movement

**TABLE 5** 

1285 NORTHERN BOULEVARD LEVEL OF SERVICE SUMMARY UNSIGNALIZED INTERSECTIONS R&M JOB No. 2021-196 JANUARY 2023

		AM COP	AM COMMUTER PEAK HOUR	HOUR	MID	MIDDAY PEAK HOUR	R	PM CON	PM COMMUTER PEAK HOUR	HOUR	SATU	SATURDAY PEAK HOUR	UR
INTERSECTION	CONDITION MVMNT	MVMNT	CONTROL DELAY (SEC/VEH)	SOT	MVMNT	CONTROL DELAY (SEC/VEH)	SOT	MVMNT	CONTROL DELAY (SEC/VEH)	ros	MVMNT	CONTROL DELAY (SEC/VEH)	SOJ
	EXISTING	EBL SBLR	11.7 17.5	В	EBL	11.4 17.7	В	EBL SBLR	12.4 17.4	В	EBL	11.9 16.2	шU
NORTHERN BOULEVARD (NYS ROUTE 25A) AT NORGATE ROAD	NO BUILD	EBL SBLR	12.1 18.7	вIJ	EBL SBLR	12.4 20.2	മറ	EBL SBLR	14.3 20.8	മധ	EBL SBLR	12.9 18.0	മറ
	BUILD	EBL SBLR	12.2 18.8	ыC	EBL SBLR	12.6 20.7	ш U	EBL SBLR	14.5 21.2	шU	EBL SBLR	13.1 18.4	шU
NORTHERN BOULEVARD (NYS ROUTE 25A) AT WEST SITE DRIVEWAY	BUILD	EBL SBLR	11.7 22.6	св	EBL SBLR	12.1 26.0	ВО	EBL SBLR	13.2 30.1	ВО	EBL SBLR	12.3 30.0	ВΩ
LOS - Level of Service													

**MVMNT - Movement** 

**TABLE 6** 

1285 NORTHERN BOULEVARD ACCIDENT TYPE SUMMARY R&M JOB No. 2021-196 JANUARY 2023

DATA														L	<u>N</u>	INJURY SUMMARY	۲۲	
COLLECTED		REAR	OVER	RIGHT	LEFT	RIGHT FIXED	-	HEAD	SIDE	DEDESTRIAN BICYCLE ANIMAL OTHER		OTHER		TOTAL	FATAL	INJURY	PROPERTY	-NON-
NOXL		END	TAKE	ANGLE	TURN	TURN OBJECT	OBJECT	NO	SWIPE					)	ACCIDENT	ACCIDENT	DAMAGE	REPORTABLE
	NORTHERN BOULEVARD						<u> </u>		<u> </u>			<u> </u>						
1/1/2017	(NYS ROUTE 25A)																	
ТО	АТ	38	16	9	e	4						2	~	70		5	65	
12/31/2019	PLANDOME ROAD																	
	NORTHERN BOULEVARD						<u> </u>		<u> </u>			<u> </u>						
1/1/2017	(NYS ROUTE 25A)																	
ТО	АТ	9	-	7								2		11		-	10	
12/31/2019	CLAPHAM AVENUE/																	
	EAST SITE DRIVEWAY																	
1/1/2017	NORTHERN BOULEVARD																	
ТО	(NYS ROUTE 25A)	7	4	9					-			з		21		-	20	
12/31/2019	АТ																	
	NORGATE ROAD																	

### <u>APPENDIX B</u>

### TRAFFIC VOLUMES AND TRIP DISTRIBUTION/ASSIGNMENT SPREADSHEETS

1285 NORTHERN BOULEVARD

Project No. 2021-196	
JUNE 2022	
GROWTH FACTOR:	0.50%
NO. OF YEARS:	2
GROWTH RATE:	1.020

			EXISTING VOLUMES	AMBIENT NO BUILD
LOCATION	DIR	М∨МТ		VOLUME
	NB	LEFT	0	0
	IND	THROUGH	0	0
		RIGHT	0	0
NORTHERN BOULEVARD	SB	LEFT	443	452
	30	THROUGH	0	452
(NYS ROUTE 25A) AT			÷	Ŧ
,	<b>F</b> D	RIGHT	160	163
PLANDOME ROAD	EB	LEFT	149	152
		THROUGH	670	683
		RIGHT	0	0
	WB	LEFT	0	0
		THROUGH	914	932
	_	RIGHT	385	393
	NB	LEFT	38	39
		THROUGH	0	0
NORTHERN BOULEVARD (NYS ROUTE 25A) AT CLAPHAM AVENUE/ EAST SITE DRIVEWAY		RIGHT	56	57
	SB	LEFT	0	0
		THROUGH	0	0
		RIGHT	0	0
	EB	LEFT	0	0
EAST SITE DRIVEWAY		THROUGH	763	778
EAST SITE DRIVEWAT		RIGHT	10	10
	WB	LEFT	17	17
		THROUGH	1057	1078
		RIGHT	0	0
	NB	LEFT	0	0
		THROUGH	0	0
		RIGHT	0	0
NORTHERN BOULEVARD (NYS ROUTE 25A) AT	SB	LEFT	9	9
		THROUGH	0	0
		RIGHT	94	96
NORGATE ROAD	EB	LEFT	42	43
		THROUGH	764	779
		RIGHT	0	0
	WB	LEFT	0	0
		THROUGH	1082	1104
		RIGHT	13	13

AM COMMUTER PEAK PERIOD			r			1			
1285 NORTHERN BOULEVARD					PROPOSED			PROPOSED	
Project No. 2021-196 JUNE 2022			PASS-BY% 0%		MEDICAL OFFICE	PASS-BY%		MIXED-USE DEVELOPMENT	SUBTOTAL TRAFFIC
					162,750 SF			14,194 SF	
					,			11,101 01	
OTHER									GENERATED
PLANNED									BY OTHER
PROJECTS					VOL			VOL	PLANNED
				ENTER	399		ENTER	20	PROJECTS
				EXIT	106		EXIT	4	
				TOTAL	505	-	TOTAL	24	-
					1			1	SUBTOTAL
					VOL			VOL	VOL
LOCATION	DIR	м∨мт	%EN	%EX		%EN	%EX		
	NB	LEFT			0			0	0
		THROUGH			0			0	0
		RIGHT			0			0	0
NORTHERN BOULEVARD	SB	LEFT	10		40	10		2	42
(NYS ROUTE 25A)		THROUGH			0			0	0
AT		RIGHT			0			0	0
PLANDOME ROAD	EB	LEFT	05		0	45		0	0
		THROUGH	35		140	45		9	149
		RIGHT			0			0	0
	WB	LEFT		25	0 37		45	0	0
		THROUGH RIGHT		35 10	11		45 10	2	39 11
		RIGHT		10			10	0	11
	NB	LEFT			0			0	0
		THROUGH			0			0	0
		RIGHT			0			0	0
NORTHERN BOULEVARD	SB	LEFT			0			0	0
(NYS ROUTE 25A)		THROUGH			0			0	0
AT		RIGHT			0			0	0
CLAPHAM AVENUE/	EB	LEFT			0			0	0
EAST SITE DRIVEWAY		THROUGH	35		140	45		9	149
		RIGHT			0	ļ		0	0
	WB	LEFT			0			0	0
		THROUGH		35	37		45	2	39
		RIGHT			0			0	0
	NB	LEFT			0			0	0
		THROUGH			0	1		0	0
		RIGHT			0	1		0	0
NORTHERN BOULEVARD	SB	LEFT			0		1 1	0	0
(NYS ROUTE 25A)		THROUGH			0			0	0
AT		RIGHT			0	1		0	0
NORGATE ROAD	EB	LEFT			0			0	0
		THROUGH	35		140	45		9	149
		RIGHT			0			0	0
	WB	LEFT			0			0	0
		THROUGH		35	37		45	2	39
		RIGHT			0			0	0

AM COMMUTER PEAK PERIOD 1285 NORTHERN BOULEVARD

Project No. 2021-196 JUNE 2022

				SUBTOTAL TRAFFIC GENERATED BY	
			AMBIENT	OTHER	TOTAL
			NO BUILD	PROJECTS	NO BUILD
LOCATION	DIR	MVMT	VOLUME		VOLUME
				-	
	NB	LEFT	0	0	0
		THROUGH	0	0	0
	0.0	RIGHT	0	÷	0
	SB	LEFT	452	42	494
(NYS ROUTE 25A)		THROUGH	0		0
	<b>F</b> D	RIGHT	163	0	163
PLANDOME ROAD	EB	LEFT	152	0	152
		THROUGH	683	149	832
	WB	RIGHT LEFT	0	0	0
	VVD		932	39	971
		RIGHT	393		404
		RIGHT	393		404
	NB	LEFT	39	0	39
	ND	THROUGH	0		0
		RIGHT	57	0	57
NORTHERN BOULEVARD	SB	LEFT	0		0
(NYS ROUTE 25A)	00	THROUGH	0	0	0
AT		RIGHT	0	0	0
CLAPHAM AVENUE/	EB	LEFT	0	0	0
EAST SITE DRIVEWAY		THROUGH	778	149	927
		RIGHT	10	0	10
	WB	LEFT	17	0	17
		THROUGH	1078	39	1117
		RIGHT	0	0	0
	NB	LEFT	0	0	0
		THROUGH	0	0	0
		RIGHT	0	0	0
NORTHERN BOULEVARD	SB	LEFT	9	0	9
(NYS ROUTE 25A)		THROUGH	0	0	0
AT		RIGHT	96	0	96
NORGATE ROAD	EB	LEFT	43	0	43
		THROUGH	779	149	928
		RIGHT	0	0	0
	WB	LEFT	0	÷	0
		THROUGH	1104	39	1143
		RIGHT	13	0	13

AM COMMUTER PEAK PERIOD 1285 NORTHERN BOULEVARD Project No. 2021-196 JUNE 2022			PASS-BY%	ENTER EXIT TOTAL	PROPOSED AUTOMOBILE SALES (NEW) 26,741 SF VOL 36 14 50	SUBTOTAL TRAFFIC GENERATED
LOCATION	DIR	MVMT	%EN	%EX	VOL	SUBTOTAL VOL
	NB	LEFT			0	0
	IND	THROUGH			0	0
		RIGHT			0	0
NORTHERN BOULEVARD	SB	LEFT			0	0
(NYS ROUTE 25A)		THROUGH			0	0
AT		RIGHT	10		4	4
PLANDOME ROAD	EB	LEFT	├	10	1	1
		THROUGH RIGHT		55	8 0	8 0
	WB	LEFT			0	0
	WB	THROUGH	55		20	20
		RIGHT			0	0
	NB	LEFT			0	0
		THROUGH			0	0
	0.5	RIGHT			0	0
	SB	LEFT			0	0
(NYS ROUTE 25A) AT		THROUGH RIGHT			0	0
CLAPHAM AVENUE/	EB	LEFT	10		4	4
EAST SITE DRIVEWAY		THROUGH		65	9	9
		RIGHT			0	0
	WB	LEFT			0	0
		THROUGH	15		5	5
	_	RIGHT	50		18	18
	NB	LEFT			0	0
		THROUGH			0	0
		RIGHT			0	0
NORTHERN BOULEVARD	SB	LEFT			0	0
(NYS ROUTE 25A)		THROUGH			0	0
AT		RIGHT			0	0
NORGATE ROAD	EB	LEFT			0	0
		THROUGH RIGHT	35		<u>13</u> 0	13 0
	WB	LEFT	├		0	0
		THROUGH	<u> </u>	35	5	5
		RIGHT			0	0
	NB	LEFT			0	0
		THROUGH			0	0
	05	RIGHT		05	0	0
NORTHERN BOULEVARD	SB	LEFT THROUGH	├	65	9	9
(NYS ROUTE 25A) AT		RIGHT	├	35	5	5
WEST SITE DRIVEWAY	EB	LEFT	25	55	9	9
		THROUGH	10		4	4
		RIGHT			0	0
	WB	LEFT			0	0
		THROUGH			0	0
		RIGHT	15		5	5

# **R&M Engineering** AM COMMUTER PEAK PERIOD 1285 NORTHERN BOULEVARD

LOCATION	DIR	мумт	TOTAL NO BUILD VOLUME	TRAFFIC GENERATED BY PROPOSED PROJECT	TOTAL BUILD VOLUME
	NB	LEFT	0	0	0
		THROUGH	0	0	0
		RIGHT	0	0	0
NORTHERN BOULEVARD	SB	LEFT	494	0	494
(NYS ROUTE 25A)		THROUGH	0	0	0
AT		RIGHT	163	4	167
PLANDOME ROAD	EB	LEFT	152	1	153
		THROUGH	832	8	840
		RIGHT	0	0	0
	WB	LEFT	0	0	0
		THROUGH	971	20	991
		RIGHT	404	0	404
	NB	LEFT	39	0	39
		THROUGH	0	0	0
		RIGHT	57	0	57
NORTHERN BOULEVARD	SB	LEFT	0	0	0
(NYS ROUTE 25A)		THROUGH	0	0	0
AT		RIGHT	0	0	0
CLAPHAM AVENUE/	EB	LEFT	0	4	4
EAST SITE DRIVEWAY		THROUGH	927	9	936
		RIGHT	10	0	10
	WB	LEFT	17	0	17
		THROUGH	1117	5	1122
		RIGHT	0	18	18
	NB	LEFT	0	0	0
		THROUGH	0	0	0
		RIGHT	0	0	0
NORTHERN BOULEVARD	SB	LEFT	9	0	9
(NYS ROUTE 25A)		THROUGH	0	0	0
AT		RIGHT	96	0	96
NORGATE ROAD	EB	LEFT	43	0	43
		THROUGH	928	13	941
		RIGHT	0	0	0
	WB	LEFT	0	0	0
		THROUGH	1143	5	1148
		RIGHT	13	0	13
	ND	LEET	0	0	0
	NB	LEFT	0	0	0 0
		THROUGH RIGHT	0	0	0
NORTHERN BOULEVARD	SB	LEFT	0	9	9
(NYS ROUTE 25A)	00	THROUGH	0	0	9 0
AT		RIGHT	0	5	5
WEST SITE DRIVEWAY	EB	LEFT	0	9	9
		THROUGH	937	4	941
		RIGHT	0	0	0.11
	WB	LEFT	0	0	0
		THROUGH	1156	0	1156
		RIGHT	0	5	5

1285 NORTHERN BOULEVARD

Project No. 2021-196	
JUNE 2022	
GROWTH FACTOR:	0.50%
NO. OF YEARS:	2
GROWTH RATE:	1.020

LOCATION	DIR	MVMT	EXISTING VOLUMES	AMBIENT NO BUILD VOLUME
	NB	LEFT	0	0
		THROUGH	0	0
		RIGHT	0	0
NORTHERN BOULEVARD	SB	LEFT	457	466
(NYS ROUTE 25A)		THROUGH	0	0
AT		RIGHT	172	175
PLANDOME ROAD	EB	LEFT	163	166
		THROUGH	1070	1091
		RIGHT	0	0
	WB	LEFT	0	0
		THROUGH	863	880
		RIGHT	235	240
	NB	LEFT	31	32
		THROUGH	0	0
		RIGHT	59	60
NORTHERN BOULEVARD	SB	LEFT	0	0
(NYS ROUTE 25A)		THROUGH	0	0
` AT		RIGHT	0	0
CLAPHAM AVENUE/	EB	LEFT	0	0
EAST SITE DRIVEWAY		THROUGH	1174	1197
		RIGHT	26	27
	WB	LEFT	25	26
		THROUGH	1010	1030
		RIGHT	0	0
	NB	LEFT	0	0
		THROUGH	0	0
		RIGHT	0	0
NORTHERN BOULEVARD	SB	LEFT	11	11
(NYS ROUTE 25A)		THROUGH	0	0
AT		RIGHT	83	85
NORGATE ROAD	EB	LEFT	45	46
		THROUGH	1189	1213
		RIGHT	0	0
	WB	LEFT	0	0
		THROUGH	1024	1044
		RIGHT	17	17

MIDDAY PEAK PERIOD						1			1
1285 NORTHERN BOULEVARD					PROPOSED			PROPOSED	
					MEDICAL OFFICE			MIXED-USE DEVELOPMENT	
Project No. 2021-196			PASS-B	/%	162,750 SF	PASS-B	Y%	14,194 SF	SUBTOTAL
JUNE 2022			0%		- ,	0%		, -	TRAFFIC
OTHER									GENERATED
PLANNED									BY OTHER
PROJECTS					VOL			VOL	PLANNED
				ENTER	325		ENTER	47	PROJECTS
				EXIT	261		EXIT	44	
				TOTAL	586	-	TOTAL	91	
					1			1	SUBTOTAL
					VOL			VOL	VOL
LOCATION	DIR	мумт	%EN	%EX		%EN	%EX		
	NB	LEFT			0	ł	┟──┤	0	0
		THROUGH	I		0		┥ ┥	0	0
NORTHERN BOULEVARD	SB	RIGHT	10		0	10		0 5	0
	5B	LEFT THROUGH	10		33 0	10		<u> </u>	38 0
(NYS ROUTE 25A)		RIGHT			0			0	0
AT PLANDOME ROAD	EB	LEFT			0			0	0
FLANDOME ROAD	ED	THROUGH	35		114	45		21	135
		RIGHT	- 35		0	40		0	0
	WB	LEFT			0			0	0
	VVD	THROUGH		35	91		45	20	111
		RIGHT		10	26		10	4	30
	NB	LEFT			0			0	0
		THROUGH			0			0	0
		RIGHT			0			0	0
NORTHERN BOULEVARD	SB	LEFT			0			0	0
(NYS ROUTE 25A)		THROUGH			0			0	0
	50	RIGHT			0			0	0
CLAPHAM AVENUE/	EB	LEFT	25		0 114	45		0	0
EAST SITE DRIVEWAY	-	THROUGH RIGHT	35		0	45		21 0	135 0
	WB	LEFT			0			0	0
	VVD	THROUGH		35	91		45	20	111
		RIGHT			0		45	0	0
						1			
	NB	LEFT			0			0	0
		THROUGH			0			0	0
		RIGHT	ļ		0			0	0
NORTHERN BOULEVARD	SB	LEFT			0		$ \downarrow  \downarrow$	0	0
(NYS ROUTE 25A)		THROUGH	<u> </u>		0	<u> </u>		0	0
		RIGHT	Į		0		┥ ┥	0	0
NORGATE ROAD	EB	LEFT	0.5		0	4-	┥ ┥	0	0
		THROUGH	35		114	45	┥	21	135
		RIGHT	l		0		┟──┤	0	0
	WB	LEFT	l	25	0		45	0	0
		THROUGH	<b>├</b> ──	35	91		45	20	111
		RIGHT			0	I		0	0

1285 NORTHERN BOULEVARD

				SUBTOTAL TRAFFIC GENERATED BY	
			AMBIENT	OTHER PROJECTS	TOTAL NO BUILD
LOCATION	DIR	м∨мт	VOLUME	PROJECTS	VOLUME
			1020112		1020112
	NB	LEFT	0	0	0
		THROUGH	0	0	0
		RIGHT	0	0	0
NORTHERN BOULEVARD	SB	LEFT	466	38	504
(NYS ROUTE 25A)		THROUGH	0	0	0
AT		RIGHT	175	0	175
PLANDOME ROAD	EB	LEFT	166	0	166
		THROUGH	1091	135	1226
		RIGHT	0	0	0
	WB	LEFT	0	0	0
		THROUGH	880	111	991
		RIGHT	240	30	270
	NB	LEFT	32	0	32
		THROUGH	0	÷	0
	0.0	RIGHT	60	0	60
	SB	LEFT	0	0	0 0
(NYS ROUTE 25A) AT		THROUGH RIGHT	0	0	0
CLAPHAM AVENUE/	EB	LEFT	0	0	0
EAST SITE DRIVEWAY		THROUGH	1197	135	1332
EAST SITE DIVIVEWAT		RIGHT	27	0	27
	WB	LEFT	26	0	26
	VVD	THROUGH	1030	111	1141
		RIGHT	0	0	0
				Ĵ	
	NB	LEFT	0	0	0
		THROUGH	0	0	0
		RIGHT	0	0	0
NORTHERN BOULEVARD	SB	LEFT	11	0	11
(NYS ROUTE 25A)		THROUGH	0	0	0
AT		RIGHT	85	0	85
NORGATE ROAD	EB	LEFT	46	0	46
		THROUGH	1213	135	1348
		RIGHT	0	0	0
	WB	LEFT	0	÷	0
		THROUGH	1044	111	1155
		RIGHT	17	0	17

1285 NORTHERN BOULEVARD					PROPOSED AUTOMOBILE SALES (NEW)	
Project No. 2021-196 JUNE 2022			PASS-BY%	)	26,741 SF	
						SUBTOTAL TRAFFIC
				Г	VOL	GENERATED
				ENTER	35	
				EXIT	41	
				TOTAL	76	
						SUBTOTAL
				0/ <b>=</b> )/	VOL	VOL
LOCATION	DIR	MVMT	%EN	%EX		
	NB	LEFT			0	0
		THROUGH			0	0
	0.0	RIGHT			0	0
NORTHERN BOULEVARD (NYS ROUTE 25A)	SB	LEFT THROUGH			0	0
AT		RIGHT	10		4	4
PLANDOME ROAD	EB	LEFT		10	4	4
		THROUGH		40	16	16
		RIGHT			0	0
	WB	LEFT	40		0 14	0
		THROUGH RIGHT	40		0	14 0
		NGITI			0	0
	NB	LEFT			0	0
		THROUGH			0	0
	-	RIGHT			0	0
	SB	LEFT			0	0
(NYS ROUTE 25A) AT		THROUGH RIGHT			0	0
CLAPHAM AVENUE/	EB	LEFT	10		4	4
EAST SITE DRIVEWAY		THROUGH		50	21	21
		RIGHT			0	0
	WB	LEFT			0	0
		THROUGH RIGHT	15 35		5 12	5 12
		RIGHT			12	12
	NB	LEFT			0	0
		THROUGH			0	0
		RIGHT			0	0
	SB	LEFT			0	0
(NYS ROUTE 25A) AT		THROUGH			0	0
NORGATE ROAD	EB	LEFT			0	0
-		THROUGH	50		18	18
		RIGHT			0	0
	WB	LEFT	ļ	50	0	0
		THROUGH RIGHT		50	21 0	21 0
	+				v	<u> </u>
	NB	LEFT			0	0
		THROUGH			0	0
	05	RIGHT			0	0
NORTHERN BOULEVARD	SB			50	21	21
(NYS ROUTE 25A) AT		THROUGH RIGHT		50	0 21	0 21
WEST SITE DRIVEWAY	EB	LEFT	40	00	14	14
		THROUGH	10		4	4
		RIGHT			0	0
	WB	LEFT			0	0
		THROUGH	45		0	0
		RIGHT	15		5	5

# **R&M Engineering MIDDAY PEAK PERIOD** 1285 NORTHERN BOULEVARD

LOCATION	DIR	м∨мт	TOTAL NO BUILD VOLUME	TRAFFIC GENERATED BY PROPOSED PROJECT	TOTAL BUILD VOLUME
	NB	LEFT	0	0	0
		THROUGH	0	0	0
		RIGHT	0	0	0
NORTHERN BOULEVARD	SB	LEFT	504	0	504
(NYS ROUTE 25A)		THROUGH	0	0	0
		RIGHT	175	4	179
PLANDOME ROAD	EB	LEFT	166	4	170
		THROUGH	1226 0	16 0	1242 0
	WB	LEFT	0	0	0
	VVD	THROUGH	991	14	1005
		RIGHT	270	0	270
		NOT I	210	0	210
	NB	LEFT	32	0	32
		THROUGH	0	0	0
		RIGHT	60	0	60
NORTHERN BOULEVARD	SB	LEFT	0	0	0
(NYS ROUTE 25A)		THROUGH	0	0	0
AT		RIGHT	0	0	0
CLAPHAM AVENUE/	EB	LEFT	0	4	4
EAST SITE DRIVEWAY		THROUGH	1332	21	1353
		RIGHT	27	0	27
	WB	LEFT	26	0	26
		THROUGH	1141	5	1146
		RIGHT	0	12	12
	NID		0	0	0
	NB	LEFT THROUGH	0	0	0
		RIGHT	0	0	0
NORTHERN BOULEVARD	SB	LEFT	11	0	11
(NYS ROUTE 25A)	00	THROUGH	0	0	0
AT		RIGHT	85	0	85
NORGATE ROAD	EB	LEFT	46	0	46
		THROUGH	1348	18	1366
		RIGHT	0	0	0
	WB	LEFT	0	0	0
		THROUGH	1155	21	1176
		RIGHT	17	0	17
	NB	LEFT	0	0	0
		THROUGH	0	0	0
	0.5	RIGHT	0	0	0
	SB	LEFT	0	21	21
(NYS ROUTE 25A) AT		THROUGH	0	0 21	0 21
WEST SITE DRIVEWAY	EB	LEFT	0	14	14
		THROUGH	1359	4	1363
		RIGHT	0	4	0
	WB	LEFT	0	0	0
		THROUGH	1173	0	1173
		RIGHT	0	5	5

## **R&M Engineering** PM COMMUTER PEAK PERIOD

1285 NORTHERN BOULEVARD

Project No. 2021-196	
JUNE 2022	
GROWTH FACTOR:	0.50%
NO. OF YEARS:	2
GROWTH RATE:	1.020

			EXISTING VOLUMES	AMBIENT NO BUILD
LOCATION	DIR	MVMT		VOLUME
	NB	LEFT	0	0
		THROUGH	0	0
		RIGHT	0	0
NORTHERN BOULEVARD	SB	LEFT	470	479
(NYS ROUTE 25A)		THROUGH	0	0
AT		RIGHT	163	166
PLANDOME ROAD	EB	LEFT	180	184
		THROUGH	1337	1364
		RIGHT	0	0
	WB	LEFT	0	0
		THROUGH	1024	1044
		RIGHT	274	279
	NB	LEFT	18	18
		THROUGH	0	0
		RIGHT	25	26
NORTHERN BOULEVARD	SB	LEFT	0	0
(NYS ROUTE 25A)		THROUGH	0	0
AT		RIGHT	0	0
CLAPHAM AVENUE/	EB	LEFT	0	0
EAST SITE DRIVEWAY		THROUGH	1492	1522
		RIGHT	14	14
	WB	LEFT	16	16
	110	THROUGH	1171	1194
		RIGHT	0	0
			0	U
	NB	LEFT	0	0
		THROUGH	0	0
		RIGHT	0	0
NORTHERN BOULEVARD	SB	LEFT	4	4
(NYS ROUTE 25A)	55	THROUGH	0	0
AT		RIGHT	89	91
NORGATE ROAD	EB	LEFT	60	61
NORGATE ROAD	ED	THROUGH	1502	1532
		RIGHT	0	0
	WB	LEFT	0	0
	VVB		÷	÷
		THROUGH	1165	1188
		RIGHT	24	24

### **R&M Engineering** PM COMMUTER PEAK PERIOD

PM COMMUTER PEAK PERIOD			r			T			1
1285 NORTHERN BOULEVARD					PROPOSED			PROPOSED	
					MEDICAL OFFICE			MIXED-USE DEVELOPMENT	
Project No. 2021-196			PASS-B	<b>ŕ</b> %	162,750 SF	PASS-B	Y%	14,194 SF	SUBTOTAL
JUNE 2022			0%		,	0%		,	TRAFFIC
OTHER									GENERATED
PLANNED									BY OTHER
PROJECTS					VOL			VOL	PLANNED
				ENTER	192		ENTER	62	PROJECTS
				EXIT	448		EXIT	51	
				TOTAL	640		TOTAL	113	
					1			1	SUBTOTAL
					VOL			VOL	VOL
LOCATION	DIR	MVMT	%EN	%EX		%EN	%EX		
	NB	LEFT			0			0	0
		THROUGH			0			0	0
		RIGHT			0			0	0
NORTHERN BOULEVARD	SB	LEFT	10		19	10		6	25
(NYS ROUTE 25A)		THROUGH			0			0	0
AT		RIGHT			0			0	0
PLANDOME ROAD	EB	LEFT	0.5		0	45		0	0
		THROUGH	35		67	45		28	95
		RIGHT			0			0	0
	WB	LEFT THROUGH		35	157		45	23	180
		RIGHT		10	45		45 10	5	50
	-	RIGHT		10	40		10	5	50
	NB	LEFT			0			0	0
		THROUGH			0			0	0
		RIGHT			0			0	0
NORTHERN BOULEVARD	SB	LEFT			0			0	0
(NYS ROUTE 25A)		THROUGH			0			0	0
AT		RIGHT			0			0	0
CLAPHAM AVENUE/	EB	LEFT			0			0	0
EAST SITE DRIVEWAY		THROUGH	35		67	45		28	95
		RIGHT			0			0	0
	WB	LEFT			0			0	0
		THROUGH		35	157		45	23	180
		RIGHT			0			0	0
	NB	LEFT			0			0	0
		THROUGH	1		0	1		0	0
		RIGHT	1		0	1		0	0
NORTHERN BOULEVARD	SB	LEFT			0			0	0
(NYS ROUTE 25A)		THROUGH			0			0	0
AT		RIGHT			0			0	0
NORGATE ROAD	EB	LEFT			0			0	0
		THROUGH	35		67	45		28	95
		RIGHT			0			0	0
	WB	LEFT			0			0	0
		THROUGH		35	157		45	23	180
		RIGHT			0			0	0

## **R&M Engineering** PM COMMUTER PEAK PERIOD

1285 NORTHERN BOULEVARD

				SUBTOTAL TRAFFIC GENERATED BY	
			AMBIENT NO BUILD	OTHER PROJECTS	TOTAL NO BUILD
LOCATION	DIR	М∨МТ	VOLUME		VOLUME
	NB	LEFT	0	0	0
		THROUGH	0	0	0
		RIGHT	0	0	0
NORTHERN BOULEVARD	SB	LEFT	479	25	504
(NYS ROUTE 25A)		THROUGH	0	0	0
AT		RIGHT	166	0	166
PLANDOME ROAD	EB	LEFT	184	0	184
		THROUGH	1364	95	1459
		RIGHT	0	0	0
	WB	LEFT	0	0	0
		THROUGH	1044	180	1224
		RIGHT	279	50	329
	NB	LEFT	18	0	18
		THROUGH	0	0	0
		RIGHT	26	0	26
NORTHERN BOULEVARD	SB	LEFT	0	0	0
(NYS ROUTE 25A)		THROUGH	0	0	0
AT		RIGHT	0	0	0
CLAPHAM AVENUE/	EB	LEFT	0	0	0
EAST SITE DRIVEWAY		THROUGH	1522	95	1617
		RIGHT	14	0	14
	WB	LEFT	16	0	16
		THROUGH	1194	180	1374
		RIGHT	0	0	0
	NB	LEFT	0	0	0
		THROUGH	0	0	0
		RIGHT	0	0	0
NORTHERN BOULEVARD	SB	LEFT	4	0	4
(NYS ROUTE 25A)		THROUGH	0	0	0
AT		RIGHT	91	0	91
NORGATE ROAD	EB	LEFT	61	0	61
		THROUGH	1532	95	1627
	14/-	RIGHT	0	0	0
	WB	LEFT	0	÷	0
		THROUGH	1188	180	1368
		RIGHT	24	0	24

Project No. 2021-196 JUNE 2022			PASS-BY%	ENTER EXIT TOTAL	PROPOSED AUTOMOBILE SALES (NEW) 26,741 SF VOL 26 39 65	SUBTOTAL TRAFFIC GENERATED
						1
LOCATION	DIR	MVMT	%EN	%EX	VOL	SUBTOTAL VOL
	NB	LEFT			0	0
		THROUGH			0	0
		RIGHT			0	0
	SB	LEFT			0	0
(NYS ROUTE 25A) AT		THROUGH RIGHT	10		0 3	03
PLANDOME ROAD	EB	LEFT	10	10	4	4
		THROUGH		35	14	14
		RIGHT			0	0
	WB	LEFT			0	0
		THROUGH	35		9	9
		RIGHT			0	0
	NB	LEFT			0	0
	IND	THROUGH			0	0
		RIGHT			0	0
NORTHERN BOULEVARD	SB	LEFT			0	0
(NYS ROUTE 25A)		THROUGH			0	0
À AT Í		RIGHT			0	0
CLAPHAM AVENUE/	EB	LEFT	10		3	3
EAST SITE DRIVEWAY		THROUGH		45	18	18
		RIGHT			0	0
	WB	LEFT	45		0 4	0
		THROUGH RIGHT	15 30		8	4 8
		RIGHT	30		0	0
	NB	LEFT			0	0
		THROUGH			0	0
		RIGHT			0	0
NORTHERN BOULEVARD	SB	LEFT			0	0
(NYS ROUTE 25A)		THROUGH			0	0
	ГР	RIGHT	├		0	0
NORGATE ROAD	EB	LEFT THROUGH	55		<u> </u>	0 14
		RIGHT	55		0	0
	WB	LEFT			0	0
		THROUGH		55	21	21
		RIGHT			0	0
		. = = =				
	NB	LEFT			0	0
		THROUGH			0	0
NORTHERN BOULEVARD	SB	RIGHT LEFT	├	45	0 18	0 18
(NYS ROUTE 25A)	00	THROUGH	<u> </u>	-10	0	0
AT		RIGHT		55	21	21
WEST SITE DRIVEWAY	EB	LEFT	45		12	12
		THROUGH	10		3	3
		RIGHT			0	0
	WB	LEFT			0	0
		THROUGH	45		0	0
		RIGHT	15		4	4

# **R&M Engineering** PM COMMUTER PEAK PERIOD 1285 NORTHERN BOULEVARD

LOCATION	DIR	MVMT	TOTAL NO BUILD VOLUME	TRAFFIC GENERATED BY PROPOSED PROJECT	TOTAL BUILD VOLUME
	NB	LEFT	0	0	0
		THROUGH	0	0	0
		RIGHT	0	0	0
	SB	LEFT	504	0	504
(NYS ROUTE 25A) AT		THROUGH RIGHT	0	0	0 169
PLANDOME ROAD	EB	LEFT	166 184	4	189
F LANDOWIE ROAD	LD	THROUGH	1459	4	1473
		RIGHT	0	0	0
	WB	LEFT	0	0	0
		THROUGH	1224	9	1233
		RIGHT	329	0	329
	NB	LEFT	18	0	18
		THROUGH	0	0	0
		RIGHT	26	0	26
NORTHERN BOULEVARD	SB	LEFT	0	0	0
(NYS ROUTE 25A)		THROUGH	0	0	0
AT		RIGHT	0	0	0
CLAPHAM AVENUE/	EB	LEFT	0	3	3
EAST SITE DRIVEWAY		THROUGH	1617	18	1635
		RIGHT	14	0	14
	WB	LEFT THROUGH	16 1374	0	16 1378
		RIGHT	0	8	8
		Riom	0	0	0
	NB	LEFT	0	0	0
		THROUGH	0	0	0
		RIGHT	0	0	0
NORTHERN BOULEVARD	SB	LEFT	4	0	4
(NYS ROUTE 25A)		THROUGH	0	0	0
AT		RIGHT	91	0	91
NORGATE ROAD	EB	LEFT	61	0	61
		THROUGH	1627	14	1641
	14/5	RIGHT	0	0	0
	WB	LEFT	0	0	0
		THROUGH	1368 24	21 0	1389 24
	_	RIGHT	24	0	24
	NB	LEFT	0	0	0
		THROUGH	0	0	0
		RIGHT	0	0	0
NORTHERN BOULEVARD	SB	LEFT	0	18	18
(NYS ROUTE 25A)		THROUGH	0	0	0
AT		RIGHT	0	21	21
WEST SITE DRIVEWAY	EB	LEFT	0	12	12
		THROUGH	1631	3	1634
		RIGHT	0	0	0
	WB	LEFT	0	0	0
		THROUGH	1393	0	1393
		RIGHT	0	4	4

1285 NORTHERN BOULEVARD

Project No. 2021-196	
JUNE 2022	
GROWTH FACTOR:	0.50%
NO. OF YEARS:	2
GROWTH RATE:	1.020

			EXISTING VOLUMES	AMBIENT NO BUILD
LOCATION	DIR	MVMT		VOLUME
	NB	LEFT	0	0
		THROUGH	0	0
		RIGHT	0	0
NORTHERN BOULEVARD	SB	LEFT	352	359
(NYS ROUTE 25A)		THROUGH	0	0
AT		RIGHT	198	202
PLANDOME ROAD	EB	LEFT	191	195
		THROUGH	1194	1218
		RIGHT	0	0
	WB	LEFT	0	0
		THROUGH	894	912
		RIGHT	273	278
	NB	LEFT	11	11
		THROUGH	0	0
		RIGHT	11	11
NORTHERN BOULEVARD	SB	LEFT	0	0
(NYS ROUTE 25A)		THROUGH	0	0
ÂT		RIGHT	0	0
CLAPHAM AVENUE/	EB	LEFT	0	0
EAST SITE DRIVEWAY		THROUGH	1374	1401
		RIGHT	9	9
	WB	LEFT	9	9
		THROUGH	1083	1105
		RIGHT	0	0
		-	-	-
	NB	LEFT	0	0
		THROUGH	0	0
		RIGHT	0	0
NORTHERN BOULEVARD	SB	LEFT	5	5
(NYS ROUTE 25A)	-	THROUGH	0	0
AT		RIGHT	67	68
NORGATE ROAD	EB	LEFT	63	64
		THROUGH	1378	1406
		RIGHT	0	0
	WB	LEFT	0	0
		THROUGH	1082	1104
		RIGHT	12	12

SATURDAY PEAK PERIOD						1			
1285 NORTHERN BOULEVARD					PROPOSED			PROPOSED	
					MEDICAL OFFICE			MIXED-USE DEVELOPMENT	
Project No. 2021-196			PASS-B)	(%	162,750 SF	PASS-B	Y%	14,194 SF	SUBTOTAL
JUNE 2022			0%		.02,100 01	0%		,	TRAFFIC
OTHER			-			-			GENERATED
PLANNED									BY OTHER
PROJECTS					VOL		Γ	VOL	PLANNED
				ENTER	280		ENTER	75	PROJECTS
				EXIT	212		EXIT	62	
				TOTAL	492	_	TOTAL	137	
	1				1		<u> </u>	1	SUBTOTAL
					VOL			VOL	VOL
LOCATION	DIR	мумт	%EN	%EX	VOL	%EN	%EX	VOL	VOL
	2		/0	/0_/(		/0=11	/0_/1		
	NB	LEFT			0			0	0
		THROUGH			0			0	0
		RIGHT			0			0	0
NORTHERN BOULEVARD	SB	LEFT	10		28	10		8	36
(NYS ROUTE 25A)		THROUGH			0			0	0
AT		RIGHT			0			0	0
PLANDOME ROAD	EB	LEFT			0			0	0
		THROUGH	35		98	45		34	132
		RIGHT			0			0	0
	WB	LEFT			0			0	0
		THROUGH		35	74		45	28	102
		RIGHT		10	21		10	6	27
	NB	LEFT			0			0	0
	IND	THROUGH			0			0	0
		RIGHT			0			0	0
NORTHERN BOULEVARD	SB	LEFT			0			0	0
(NYS ROUTE 25A)	00	THROUGH			0			0	0
AT		RIGHT			0			0	0
CLAPHAM AVENUE/	EB	LEFT			0			0	0
EAST SITE DRIVEWAY		THROUGH	35		98	45		34	132
		RIGHT	1		0			0	0
	WB	LEFT			0			0	0
		THROUGH		35	74		45	28	102
		RIGHT			0			0	0
	NB	LEFT			0		├	0	0
	ND	THROUGH	<del> </del>		0		┟──┤	0	0
		RIGHT			0			0	0
NORTHERN BOULEVARD	SB	LEFT			0			0	0
(NYS ROUTE 25A)		THROUGH	<u> </u>		0		├	0	0
AT		RIGHT	1		0			0	0
NORGATE ROAD	EB	LEFT	1		0	1		0	0
	<u> </u>	THROUGH	35		98	45		34	132
		RIGHT	1		0			0	0
	WB	LEFT	1		0			0	0
		THROUGH		35	74		45	28	102
		RIGHT			0			0	0

1285 NORTHERN BOULEVARD

				SUBTOTAL TRAFFIC GENERATED BY	
			AMBIENT	OTHER PROJECTS	TOTAL NO BUILD
LOCATION	DIR	м∨мт	VOLUME	INOJECIO	VOLUME
	NB	LEFT	0	0	0
		THROUGH	0	0	0
		RIGHT	0	0	0
NORTHERN BOULEVARD	SB	LEFT	359	36	395
(NYS ROUTE 25A)		THROUGH	0	0	0
	50	RIGHT	202	0	202
PLANDOME ROAD	EB	LEFT THROUGH	195 1218	0 132	195 1350
		RIGHT	0	0	0
	WB	LEFT	0	ů	0
	110	THROUGH	912	102	1014
		RIGHT	278	27	305
			2.0		
	NB	LEFT	11	0	11
		THROUGH	0	0	0
		RIGHT	11	0	11
NORTHERN BOULEVARD	SB	LEFT	0	0	0
(NYS ROUTE 25A)		THROUGH	0	0	0
AT		RIGHT	0	0	0
CLAPHAM AVENUE/	EB	LEFT	0	0	0
EAST SITE DRIVEWAY		THROUGH	1401	132	1533
		RIGHT	9	-	9
	WB	LEFT	9	0	9
		THROUGH	1105		1207
		RIGHT	0	0	0
	NB	LEFT	0	0	0
	IND	THROUGH	0	0	0
		RIGHT	0	0	0
NORTHERN BOULEVARD	SB	LEFT	5	0	5
(NYS ROUTE 25A)	00	THROUGH	0	0	0
AT		RIGHT	68	0	68
NORGATE ROAD	EB	LEFT	64	0	64
		THROUGH	1406	132	1538
		RIGHT	0	0	0
	WB	LEFT	0	0	0
		THROUGH	1104	102	1206
		RIGHT	12	0	12

Project No. 2021-196			PASS-BY%		PROPOSED AUTOMOBILE SALES (NEW) 26,741 SF	
JUNE 2022				г	VOL	SUBTOTAL TRAFFIC GENERATED
			Іг	ENTER	53	
			I F	ENTER	54	
				TOTAL	107	
			ᆝ	TOTAL	107	
LOCATION	DIR	MVMT	%EN	%EX	VOL	SUBTOTAL VOL
	NB	LEFT	1 1		0	0
		THROUGH			0	0
		RIGHT	i i		0	0
NORTHERN BOULEVARD	SB	LEFT			0	0
(NYS ROUTE 25A)		THROUGH			0	0
AT		RIGHT	10		5	5
PLANDOME ROAD	EB	LEFT		10	5	5
		THROUGH		40	22	22
		RIGHT			0	0
	WB	LEFT			0	0
		THROUGH	40		21	21
		RIGHT			0	0
	NB	LEFT			0	0
		THROUGH			0	0
		RIGHT			0	0
NORTHERN BOULEVARD	SB	LEFT			0	0
(NYS ROUTE 25A)		THROUGH			0	0
AT		RIGHT			0	0
CLAPHAM AVENUE/	EB	LEFT	10		5	5
EAST SITE DRIVEWAY		THROUGH	┥──┤	50	27	27
		RIGHT			0	0
	WB	LEFT THROUGH	15		8	8
		RIGHT	35		19	19
		NGITI			19	19
	NB	LEFT	1 1		0	0
		THROUGH	1 1		0	0
		RIGHT			0	0
NORTHERN BOULEVARD	SB	LEFT			0	0
(NYS ROUTE 25A)		THROUGH			0	0
AT		RIGHT			0	0
NORGATE ROAD	EB	LEFT			0	0
		THROUGH	50		27	27
		RIGHT			0	0
	WB	LEFT	$ \downarrow  \downarrow$		0	0
		THROUGH	<b>└───</b> ┤	50	27	27
		RIGHT	┞───┤		0	0
			┟──┤		0	
	NB		├───┤		0	0
		THROUGH RIGHT	┝──┤		0	0
NORTHERN BOULEVARD	SB	LEFT	┟──┤	50	27	27
(NYS ROUTE 25A)	55	THROUGH	<del>   </del>	50	0	0
AT		RIGHT	┼ ┤	50	27	27
WEST SITE DRIVEWAY	EB	LEFT	40		21	21
		THROUGH	10		5	5
		RIGHT			0	0
	WB	LEFT			0	0
	<u> </u>	THROUGH			0	0
		RIGHT	15		8	8
	1	1			2	-

# **R&M Engineering SATURDAY PEAK PERIOD** 1285 NORTHERN BOULEVARD

LOCATION	DIR	мумт	TOTAL NO BUILD VOLUME	TRAFFIC GENERATED BY PROPOSED PROJECT	TOTAL BUILD VOLUME
	NB	LEFT	0	0	0
		THROUGH	0	0	0
		RIGHT	0	0	0
NORTHERN BOULEVARD	SB	LEFT	395	0	395
(NYS ROUTE 25A)		THROUGH	0	0	0
AT		RIGHT	202	5	207
PLANDOME ROAD	EB	LEFT	195	5	200
		THROUGH	1350	22	1372
		RIGHT	0	0	0
	WB	LEFT	0	0	0
		THROUGH	1014	21	1035
		RIGHT	305	0	305
	NB	LEFT	11	0	11
		THROUGH	0	0	0
		RIGHT	11	0	11
NORTHERN BOULEVARD	SB	LEFT	0	0	0
(NYS ROUTE 25A)		THROUGH	0	0	0
ÂT		RIGHT	0	0	0
CLAPHAM AVENUE/	EB	LEFT	0	5	5
EAST SITE DRIVEWAY		THROUGH	1533	27	1560
		RIGHT	9	0	9
	WB	LEFT	9	0	9
		THROUGH	1207	8	1215
		RIGHT	0	19	19
	NB	LEFT	0	0	0
		THROUGH	0	0	0
	C D	RIGHT	0	0	05
	SB	LEFT	5	0	5 0
(NYS ROUTE 25A) AT		THROUGH RIGHT	68	0	68
	EB	LEFT	64	0	64
NORGATE ROAD	LD	THROUGH	1538	27	1565
		RIGHT	0	0	0
	WB	LEFT	0	0	0
		THROUGH	1206	27	1233
		RIGHT	12	0	12
	NB	LEFT	0	0	0
		THROUGH	0	0	0
		RIGHT	0	0	0
NORTHERN BOULEVARD	SB	LEFT	0	27	27
(NYS ROUTE 25A)		THROUGH	0	0	0
AT		RIGHT	0	27	27
WEST SITE DRIVEWAY	EB	LEFT	0	21	21
		THROUGH	1543	5	1548
		RIGHT	0	0	0
	WB	LEFT	0	0	0
		THROUGH	1218	0	1218
		RIGHT	0	8	8

### <u>APPENDIX C</u>

### COVID-19 ADJUSTMENT TABLES

#### **R&M ENGINEERING**

1285 NORTHERN BOULEVARD

COVID-19 ADJUSTMENT TABLES: NORTHERN BOULEVARD (NYS ROUTE 25A) AT PLANDOME ROAD R&M JOB No. 2021-196 JUNE 2022

	0047	0040
YEAR:	2017	2019
GROWTH FACTOR:	0.50%	0.50%
NO. OF YEARS:	5	3
GROWTH RATE:	1.030	1.020

A = Traffic volumes observed by R&M Engineering on Thursday, June 16, 2022 and Saturday, June 18, 2022

B = Sum of A

C = Weekday Average Daily Traffic (ADT) Axle Factored obtained from the NYSDOT (see Reference & Year)

- D = Weekday ADT elevated to year 2022
- = C x Growth Rate (see Reference & Year)

NOTE: Observed volumes that are greater than or comparable to historical data (rate of 1.05 or less) were left unadjusted.

	AM COMMUTER PEAK HOUR									
MOVEMENT	R&M	R&M TOTAL	ADT	ADT 2022	RATE	ADJ VOLUME	REFERENCE & YEAR			
	А	В	С	D	D/B	A x (D / B)				
SBL	359	489	586	604	1.23	443	NYSDOT AADT Report - Year			
SBR	130	409	000	604	1.23	160	2017 - Station #036464			
EBL	144	780	745	760	0.97	140				
EBT	636	760	745	760	0.97	620	NYSDOT AADT Report - Year			
WBT	914	1299	1021	1041	0.80	733	2019 - Station #030227			
WBR	385	1299	1021	1041	0.80	309				

	MIDDAY PEAK HOUR										
MOVEMENT	R&M	R&M TOTAL	ADT	ADT 2022	RATE	ADJ VOLUME	REFERENCE & YEAR				
	А	В	С	D	D/B	A x (D / B)					
SBL	391	538	611	629	1.17	457	NYSDOT AADT Report - Year				
SBR	147	556	011	029	1.17	172	2017 - Station #036464				
EBL	157	1208	908	926	0.77	120					
EBT	1051	1200	900	920	0.77	806	NYSDOT AADT Report - Year				
WBT	863	1098	949	968	0.88	761	2019 - Station #030227				
WBR	235	1090	549	300	0.00	207					

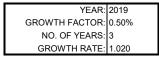
	PM COMMUTER PEAK HOUR										
MOVEMENT	R&M	R&M TOTAL	ADT	ADT 2022	RATE	ADJ VOLUME	REFERENCE & YEAR				
	А	В	С	D	D/B	A x (D / B)					
SBL	390	525	614	632	1.20	470	NYSDOT AADT Report - Year				
SBR	135	525	014	032	1.20	163	2017 - Station #036464				
EBL	173	1482	1120	1142	0.77	133					
EBT	1309	1402	1120	1142	0.77	1009	NYSDOT AADT Report - Year				
WBT	1024	1298	889	907	0.70	715	2019 - Station #030227				
WBR	274	1290	009	907	0.70	191					

	SATURDAY PEAK HOUR										
MOVEMENT	R&M	R&M TOTAL	ADT	ADT 2022	RATE	ADJ VOLUME	REFERENCE & YEAR				
	А	В	С	D	D/B	A x (D / B)					
SBL	293	458	N/A	N/A	1.20	352	Average of Weekday Rates on				
SBR	165	400	N/A	N/A	1.20	198	Southbound Approach				
EBL	191	1378	1016	1036	0.75	144					
EBT	1187	1370	1010	1030	0.75	893	NYSDOT AADT Report - Year				
WBT	894	1167	975	995	0.85	762	2019 - Station #030227				
WBR	273	1107	975	990	0.05	233					

#### **R&M ENGINEERING**

1285 NORTHERN BOULEVARD

COVID-19 ADJUSTMENT TABLES: NORTHERN BOULEVARD (NYS ROUTE 25A) AT CLAPHAM AVENUE R&M JOB No. 2021-196 JUNE 2022



A = Traffic volumes observed by R&M Engineering on Thursday, June 16, 2022 and Saturday, June 18, 2022

B = Sum of A

C = Weekday Average Daily Traffic (ADT) Axle Factored obtained from the NYSDOT (see Reference & Year)

- D = Weekday ADT elevated to year 2022
  - = C x Growth Rate (see Reference & Year)

NOTE: Observed volumes that are greater than or comparable to historical data (rate of 1.05 or less) were left unadjusted.

			AM CON	IMUTER PI	EAK HOUR	2	
MOVEMENT	R&M	R&M TOTAL	ADT	ADT 2022	RATE	ADJ VOLUME	REFERENCE & YEAR
	А	В	С	D	D/B	A x (D / B)	
NBL	15	37	93	95	2.56	38	NYSDOT AADT Report - Year
NBR	22	57	95	95	2.50	56	2019 - Station #031318
EBT	759	769	745	760	0.99	750	
EBR	10	709	745	700	0.99	10	NYSDOT AADT Report - Year
WBL	17	1054	1021	1041	0.99	17	2019 - Station #030227
WBT	1037	1054	1021	1041	0.99	1025	

			MID	DAY PEAK	HOUR		
MOVEMENT	R&M	R&M TOTAL	ADT	ADT 2022	RATE	ADJ VOLUME	REFERENCE & YEAR
	А	В	С	D	D/B	A x (D / B)	
NBL	31	90	44	45	0.50	15	NYSDOT AADT Report - Year
NBR	59	90	44	45	0.50	29	2019 - Station #031318
EBT	1157	1183	908	926	0.78	906	
EBR	26	1105	900	920	0.76	20	NYSDOT AADT Report - Year
WBL	25	1015	949	968	0.95	24	2019 - Station #030227
WBT	990	1015	349	300	0.95	944	

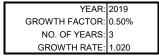
			PM CON	IMUTER PI	EAK HOUR	1	
MOVEMENT	R&M	R&M TOTAL	ADT	ADT 2022	RATE	ADJ VOLUME	REFERENCE & YEAR
	А	В	С	D	D/B	A x (D / B)	
NBL	18	43	32	33	0.76	14	NYSDOT AADT Report - Year
NBR	25	43	52		0.70	19	2019 - Station #031318
EBT	1492	1506	1120	1142	0.76	1132	
EBR	14	1500	1120	1142	0.70	11	NYSDOT AADT Report - Year
WBL	16	1107	889	907	0.82	13	2019 - Station #030227
WBT	1091	1107	009	907	0.02	894	

			SATU	RDAY PEA	K HOUR		
MOVEMENT	R&M	R&M TOTAL	ADT	ADT 2022	RATE	ADJ VOLUME	REFERENCE & YEAR
	А	В	С	D	D/B	A x (D / B)	
NBL	9	18	N/A	N/A	1.27	11	Average of Weekday Rates on
NBR	9	10	N/A	N/A	1.27	11	Southbound Approach
EBT	1374	1383	1016	1036	0.75	1030	
EBR	9	1363	1010	1030	0.75	7	NYSDOT AADT Report - Year
WBL	9	1070	975	995	0.93	8	2019 - Station #030227
WBT	1061	1070	975	995	0.93	986	

#### **R&M ENGINEERING**

1285 NORTHERN BOULEVARD

COVID-19 ADJUSTMENT TABLES: NORTHERN BOULEVARD (NYS ROUTE 25A) AT NORGATE ROAD R&M JOB No. 2021-196 JUNE 2022



A = Traffic volumes observed by R&M Engineering on Thursday, June 16, 2022 and Saturday, June 18, 2022

B = Sum of A

C = Weekday Average Daily Traffic (ADT) Axle Factored obtained from the NYSDOT (see Reference & Year)

- D = Weekday ADT elevated to year 2022
  - = C x Growth Rate (see Reference & Year)

NOTE: Observed volumes that are greater than or comparable to historical data (rate of 1.05 or less) were left unadjusted.

			AM CON	IMUTER PI	EAK HOUF	ł	
MOVEMENT	R&M	R&M TOTAL	ADT	ADT 2022	RATE	ADJ VOLUME	REFERENCE & YEAR
	А	В	С	D	D/B	A x (D / B)	
SBL	9	103	N/A	N/A		0	No Adjustments Necessary
SBR	94	103	N/A	N/A		0	No Adjustments Necessary
EBL	42	806	745	760	0.94	40	
EBT	764	800	745	760	0.94	720	NYSDOT AADT Report - Year
WBT	1043	1056	1021	1041	0.99	1029	2019 - Station #030227
WBR	13	1050	1021	1041	0.99	13	

			MID	DAY PEAK	HOUR		
MOVEMENT	R&M	R&M TOTAL	ADT	ADT 2022	RATE	ADJ VOLUME	REFERENCE & YEAR
	А	В	С	D	D/B	A x (D / B)	
SBL	11	94	N/A	N/A		0	No Adjustments Necessary
SBR	83	94	N/A	N/A		0	No Adjustments Necessary
EBL	45	1234	908	926	0.75	34	
EBT	1189	1234	908	920	0.75	892	NYSDOT AADT Report - Year
WBT	1014	1031	949	968	0.94	952	2019 - Station #030227
WBR	17	1031	949	900	0.94	16	

			PM CON	IMUTER PI	EAK HOUR	1	
MOVEMENT	R&M	R&M TOTAL	ADT	ADT 2022	RATE	ADJ VOLUME	REFERENCE & YEAR
	А	В	С	D	D/B	A x (D / B)	
SBL	4	93	N/A	N/A		0	No Adjustments Necessary
SBR	89	93	N/A	N/A		0	No Adjustments Necessary
EBL	60	1524	1120	1142	0.75	45	
EBT	1464	1524	1120	1142	0.75	1097	NYSDOT AADT Report - Year
WBT	1146	1170	889	907	0.78	888	2019 - Station #030227
WBR	24	1170	009	907	0.76	19	

			SATU	RDAY PEA	K HOUR		
MOVEMENT	R&M	R&M TOTAL	ADT	ADT 2022	RATE	ADJ VOLUME	REFERENCE & YEAR
	А	В	С	D	D/B	A x (D / B)	
SBL	5	72	N/A	N/A		0	No Adjustments Necessary
SBR	67	12	N/A	N/A		0	No Adjustments Necessary
EBL	63	1433	1016	1036	0.72	46	
EBT	1370	1455	1010	1030	0.72	991	NYSDOT AADT Report - Year
WBT	1059	1071	975	995	0.93	983	2019 - Station #030227
WBR	12	1071	975	990	0.93	11	

### <u>APPENDIX D</u>

### CAPACITY ANALYSIS WORKSHEETS

### SIGNALIZED INTERSECTIONS

	×	_	+	<b>\</b>	5	7
	-			-		-
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	<u></u>	<u></u>	<b>↑</b> Ъ		ካካ	1
Traffic Volume (vph)	149	670	914	385	443	160
Future Volume (vph)	149	670	914	385	443	160
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	110			0	435	115
Storage Lanes	1			0	1	1
Taper Length (ft)	25				25	
Lane Util. Factor	1.00	0.95	0.95	0.95	0.97	1.00
Frt			0.956			0.850
Flt Protected	0.950				0.950	
Satd. Flow (prot)	1752	3505	3351	0	3400	1568
Flt Permitted	0.950				0.950	
Satd. Flow (perm)	1752	3505	3351	0	3400	1568
Right Turn on Red				No		No
Satd. Flow (RTOR)						
Link Speed (mph)		35	35		30	
Link Distance (ft)		300	1113		956	
Travel Time (s)		5.8	21.7		21.7	
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
	3%	3%	3%	3%	3%	3%
Heavy Vehicles (%)						
Adj. Flow (vph)	154	691	942	397	457	165
Shared Lane Traffic (%)	4 - 4	004	4000	^	457	405
Lane Group Flow (vph)	154	691	1339	0	457	165
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right
Median Width(ft)		12	12		24	
Link Offset(ft)		0	0		0	
Crosswalk Width(ft)		16	16		16	
Two way Left Turn Lane		Yes				
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15			9	15	9
Number of Detectors	1	2	2	-	1	1
Detector Template	Left	Thru	Thru		Left	Right
Leading Detector (ft)	20	100	100		20	20
Trailing Detector (ft)	20	0	0		20	20
Detector 1 Position(ft)	0	0	0		0	0
Detector 1 Size(ft)	20		6		20	20
· · · · · · · · · · · · · · · · · · ·		6 Сы Бу	-			
Detector 1 Type	CI+Ex	Cl+Ex	CI+Ex		CI+Ex	CI+Ex
Detector 1 Channel		• •				
Detector 1 Extend (s)	0.0	0.0	0.0		0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0		0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0		0.0	0.0
Detector 2 Position(ft)		94	94			
Detector 2 Size(ft)		6	6			
Detector 2 Type		Cl+Ex	Cl+Ex			
Detector 2 Channel						
Detector 2 Extend (s)		0.0	0.0			
Turn Type	Prot	NA	NA		Prot	Over
Protected Phases	5	2	6		7	5
	5	2	U		'	5

June 2	2022
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	٦	-	-	*	1	1
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Permitted Phases						
Detector Phase	5	2	6		7	5
Switch Phase						
Minimum Initial (s)	8.0	10.0	10.0		8.0	8.0
Minimum Split (s)	14.5	17.0	36.0		14.5	14.5
Total Split (s)	27.0	90.0	90.0		33.0	27.0
Total Split (%)	18.0%	60.0%	60.0%		22.0%	18.0%
Maximum Green (s)	20.5	83.0	83.0		26.5	20.5
Yellow Time (s)	4.0	5.0	5.0		4.0	4.0
All-Red Time (s)	2.5	2.0	2.0		2.5	2.5
Lost Time Adjust (s)	0.0	0.0	0.0		0.0	0.0
Total Lost Time (s)	6.5	7.0	7.0		6.5	6.5
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0	3.0	3.0		3.0	3.0
Recall Mode	None	C-Max	C-Max		None	None
Walk Time (s)	7.0		7.0		7.0	7.0
Flash Dont Walk (s)	21.0		22.0		22.0	21.0
Pedestrian Calls (#/hr)	0		0		0	0
Act Effct Green (s)	19.1	86.8	86.8		24.1	19.1
Actuated g/C Ratio	0.13	0.58	0.58		0.16	0.13
v/c Ratio	0.69	0.34	0.69		0.84	0.83
Control Delay	97.4	13.4	25.3		75.0	94.6
Queue Delay	8.3	0.4	0.0		0.0	0.0
Total Delay	105.7	13.8	25.3		75.0	94.6
LOS	F	В	С		E	F
Approach Delay		30.6	25.3		80.2	
Approach LOS		С	С		F	
Intersection Summary						
,	Other					
Area Type:	Other					
Cycle Length: 150 Actuated Cycle Length: 1	50					
Offset: 14 (9%), Reference		2.EBT on		Start of V	Vallow	
	eu lo priase		u 0.VVDI,	Start of	I GIIOW	
Natural Cycle: 80	oordinated					
Control Type: Actuated-C Maximum v/c Ratio: 0.84	oorumated					
	20.1			1	torocatio	
Intersection Signal Delay:					ntersection	n LOS: D of Service
Intersection Capacity Utili Analysis Period (min) 15	zauon 75.1%	)		IC.	O Level	UI SEIVICE
niaiysis reilou (11111) 13						
	lorthern Boul	evard (N	YS Route	25A) & P	landome	Road
<b>*</b>	- (n)					
Ø5 27 s	→Ø2 (R) 90 s					

Ø5	→Ø2 (R)	•Ø7
27 s	90 s	33 s
	+	
	Ø6 (R)	
	90 s	

Lanes, Volumes, Ti 2: Clapham Avenue	•	Site Dr		/ & No	rthorn	Bouley	vard (N		outo 25	501	hu	ne 2022
					<b>4</b>			<b>†</b>		<u>,                                    </u>	Ļ	IC 2022
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	<b>≜</b> †⊅		<u> </u>	<b>≜</b> †⊅			र्भ	1			
Traffic Volume (vph)	0	763	10	17	1057	0	38	Ō	56	0	0	0
Future Volume (vph)	0	763	10	17	1057	0	38	0	56	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	90		0	130		0	170		0	0		0
Storage Lanes	1		0	1		0	1		1	0		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.998							0.850			
Flt Protected		0.000		0.950				0.950	0.000			
Satd. Flow (prot)	1845	3498	0	1752	3505	0	0	1752	1568	0	0	0
Flt Permitted	1010	0100	Ű	0.317	0000	Ŭ	Ŭ	0.950	1000	Ū	Ŭ	Ū
Satd. Flow (perm)	1845	3498	0	585	3505	0	0	1752	1568	0	0	0
Right Turn on Red	10-10	0400	Yes	000	0000	Yes	U	1102	Yes	U	0	Yes
Satd. Flow (RTOR)		1	163			163			75			103
Link Speed (mph)		35			35			30	15		30	
Link Distance (ft)		183			300			662			430	
		3.6			5.8			15.0			430 9.8	
Travel Time (s)	0.02		0.02	0.02		0.02	0.02		0.02	0.02		0.02
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%
Adj. Flow (vph)	0	820	11	18	1137	0	41	0	60	0	0	0
Shared Lane Traffic (%)	0	004	0	40	4407	0	0		00	0	^	
Lane Group Flow (vph)	0	831	0	18	1137	0	0	41	60	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane		Yes			Yes							
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2		1	2		1	2	1			
Detector Template	Left	Thru		Left	Thru		Left	Thru	Right			
Leading Detector (ft)	20	100		20	100		20	100	20			
Trailing Detector (ft)	0	0		0	0		0	0	0			
Detector 1 Position(ft)	0	0		0	0		0	0	0			
Detector 1 Size(ft)	20	6		20	6		20	6	20			
Detector 1 Type	Cl+Ex	CI+Ex		Cl+Ex	CI+Ex		CI+Ex	CI+Ex	CI+Ex			
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0			
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0			
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0			
Detector 2 Position(ft)		94			94			94				
Detector 2 Size(ft)		6			6			6				
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex				
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0				
Turn Type	Perm	NA		pm+pt	NA		Perm	NA	Perm			
Protected Phases		2		1	6			4				
		2			v			7				

Existing AM AK

Lanes, Volumes, <sup>-</sup> 2: Clapham Avenu	0	Site Dri	veway	/ & No	rthern	Boule	/ard (N		oute 25	A)	Jur	ne 2022
i	٦	-	$\mathbf{r}$	4	+	•	-	Ť	1	4	ţ	~
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Permitted Phases	2			6			4		4			
Detector Phase	2	2		1	6		4	4	4			
Switch Phase												
Minimum Initial (s)	10.0	10.0		3.0	10.0		8.0	8.0	8.0			
Minimum Split (s)	18.0	18.0		8.3	18.0		16.0	16.0	16.0			
Total Split (s)	95.0	95.0		18.0	113.0		37.0	37.0	37.0			
Total Split (%)	63.3%	63.3%		12.0%	75.3%		24.7%	24.7%	24.7%			
Maximum Green (s)	87.0	87.0		12.7	105.0		29.0	29.0	29.0			
Yellow Time (s)	5.0	5.0		3.2	5.0		4.0	4.0	4.0			
All-Red Time (s)	3.0	3.0		2.1	3.0		4.0	4.0	4.0			
Lost Time Adjust (s)	0.0	0.0		0.0	0.0			0.0	0.0			
Total Lost Time (s)	8.0	8.0		5.3	8.0			8.0	8.0			
Lead/Lag	Lag	Lag		Lead								
Lead-Lag Optimize?	Yes	Yes		Yes								
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0	3.0			
Recall Mode	C-Max	C-Max		None	C-Max		None	None	None			
Walk Time (s)	7.0	7.0					7.0	7.0	7.0			
Flash Dont Walk (s)	25.0	25.0					22.0	22.0	22.0			
Pedestrian Calls (#/hr)	0	0					0	0	0			
Act Effct Green (s)		122.6		130.4	129.3			9.5	9.5			
Actuated g/C Ratio		0.82		0.87	0.86			0.06	0.06			
v/c Ratio		0.29		0.03	0.38			0.37	0.36			
Control Delay		5.0		0.4	0.6			76.4	14.2			
Queue Delay		0.0		0.0	0.4			0.0	0.1			
Total Delay		5.0		0.4	1.0			76.4	14.2			
LOS		A		A	A			E	В			
Approach Delay		5.0			1.0			39.5				
Approach LOS		А			A			D				
Intersection Summary												
Area Type:	Other											
Cycle Length: 150												
Actuated Cycle Length: 15	50											
Offset: 39 (26%), Reference		e 2:EBTL a	and 6:WE	TL, Starl	t of Yellow	ı						
Natural Cycle: 50												
Control Type: Actuated-Co	oordinated											
Maximum v/c Ratio: 0.38												
Intersection Signal Delay:	4.5			lr	ntersectior	n LOS: A						
Intersection Capacity Utiliz	zation 49.2%	, D		10	CU Level o	of Service	Α					
Analysis Period (min) 15												

### Splits and Phases: 2: Clapham Avenue/East Site Driveway & Northern Boulevard (NYS Route 25A)



	×	_	+		1	1
	-				-	
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	<u></u>	<u></u>	<b>∱</b> }		ካካ	1
Traffic Volume (vph)	163	1070	863	235	457	172
Future Volume (vph)	163	1070	863	235	457	172
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	110			0	435	115
Storage Lanes	1			0	1	1
Taper Length (ft)	25				25	
Lane Util. Factor	1.00	0.95	0.95	0.95	0.97	1.00
Frt			0.968			0.850
Flt Protected	0.950				0.950	
Satd. Flow (prot)	1752	3505	3393	0	3400	1568
Flt Permitted	0.950			-	0.950	
Satd. Flow (perm)	1752	3505	3393	0	3400	1568
Right Turn on Red	1102	5000	5000	No	5100	No
Satd. Flow (RTOR)				NU		110
Link Speed (mph)		35	35		30	
Link Distance (ft)		300	1113		956	
Travel Time (s)	0.00	5.8	21.7	0.00	21.7	0.00
Peak Hour Factor	0.89	0.89	0.89	0.89	0.89	0.89
Heavy Vehicles (%)	3%	3%	3%	3%	3%	3%
Adj. Flow (vph)	183	1202	970	264	513	193
Shared Lane Traffic (%)						
Lane Group Flow (vph)	183	1202	1234	0	513	193
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right
Median Width(ft)		12	12		24	-
Link Offset(ft)		0	0		0	
Crosswalk Width(ft)		16	16		16	
Two way Left Turn Lane		Yes				
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	1.00	1.00	9	15	9
Number of Detectors	15	2	2	3	10	9
	Left				•	
Detector Template		Thru	Thru		Left	Right
Leading Detector (ft)	20	100	100		20	20
Trailing Detector (ft)	0	0	0		0	0
Detector 1 Position(ft)	0	0	0		0	0
Detector 1 Size(ft)	20	6	6		20	20
Detector 1 Type	CI+Ex	Cl+Ex	CI+Ex		Cl+Ex	Cl+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0		0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0		0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0		0.0	0.0
Detector 2 Position(ft)		94	94			
Detector 2 Size(ft)		6	6			
Detector 2 Type		Cl+Ex	CI+Ex			
Detector 2 Channel						
Detector 2 Extend (s)		0.0	0.0			
	Prot		NA		Prot	Over
Turn Type		NA			Prot	
Protected Phases	5	2	6		7	5

Existing Midday AK

June 2	2022
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Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Permitted Phases						
Detector Phase	5	2	6		7	5
Switch Phase						
Minimum Initial (s)	8.0	10.0	10.0		8.0	8.0
Minimum Split (s)	14.5	17.0	36.0		14.5	14.5
Total Split (s)	29.0	82.0	82.0		29.0	29.0
Total Split (%)	20.7%	58.6%	58.6%		20.7%	20.7%
Maximum Green (s)	22.5	75.0	75.0		22.5	22.5
Yellow Time (s)	4.0	5.0	5.0		4.0	4.0
All-Red Time (s)	2.5	2.0	2.0		2.5	2.5
Lost Time Adjust (s)	0.0	0.0	0.0		0.0	0.0
Total Lost Time (s)	6.5	7.0	7.0		6.5	6.5
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0	3.0	3.0		3.0	3.0
Recall Mode	None	C-Max	C-Max		None	None
Walk Time (s)	7.0		7.0		7.0	7.0
Flash Dont Walk (s)	21.0		22.0		22.0	21.0
Pedestrian Calls (#/hr)	0		0		0	0
Act Effct Green (s)	20.5	77.0	77.0		22.5	20.5
Actuated g/C Ratio	0.15	0.55	0.55		0.16	0.15
v/c Ratio	0.71	0.62	0.66		0.94	0.84
Control Delay	84.1	18.2	24.8		84.1	87.3
Queue Delay	5.9	0.2	0.0		0.0	0.0
Total Delay	90.0	18.4	24.8		84.1	87.3
LOS	F	В	С		F	F
Approach Delay		27.9	24.8		85.0	
Approach LOS		С	С		F	
Intersection Summary						
Area Type:	Other					
Cycle Length: 140						
Actuated Cycle Length: 14						
Offset: 27 (19%), Reference	ed to phase	e 2:EBT a	nd 6:WBT	, Start of	Yellow	
Natural Cycle: 80						
Control Type: Actuated-Co	ordinated					
Maximum v/c Ratio: 0.94						
Intersection Signal Delay:					ntersectio	
Intersection Capacity Utiliz	ation 70.1%	)		IC	CU Level	of Service
Analysis Period (min) 15						
Splits and Phases: 1: No	orthern Boul	evard (N	VS Route	25Δነ ይ ወ	landomo	Road
					andome	Nuau
<b>\$</b> Ø5	-•ø2	(R)				



2: Clapham Avenue	e/East S	Site Dr	iveway	/ & No	rthern	Boulev	/ard (N	IYS Ro	oute 25	δA)	Jur	ne 2022
	≯	-	$\mathbf{r}$	4	←	•	•	t	1	5	Ļ	~
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	<u>۲</u>	¢î≽		<u>۲</u>	A1⊅			<del>ا</del>	1			
Traffic Volume (vph)	0	1174	26	25	1010	0	31	Ö	59	0	0	0
Future Volume (vph)	0	1174	26	25	1010	0	31	0	59	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	90		0	130		0	170		0	0		0
Storage Lanes	1		0	1		0	1		1	0		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.997							0.850			
Flt Protected				0.950				0.950				
Satd. Flow (prot)	1845	3494	0	1752	3505	0	0	1752	1568	0	0	0
Flt Permitted				0.176				0.950				
Satd. Flow (perm)	1845	3494	0	325	3505	0	0	1752	1568	0	0	0
Right Turn on Red			Yes			Yes	-		Yes	-	-	Yes
Satd. Flow (RTOR)		2							80			
Link Speed (mph)		35			35			30	00		30	
Link Distance (ft)		183			300			662			430	
Travel Time (s)		3.6			5.8			15.0			9.8	
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Heavy Vehicles (%)	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%
Adj. Flow (vph)	0	1290	29	27	1110	0	34	0	65	0	0	0
Shared Lane Traffic (%)	0	1250	25	21	1110	U	04	U	00	U	U	U
Lane Group Flow (vph)	0	1319	0	27	1110	0	0	34	65	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)	Lon	12	rtigrit	Lon	12	rugin	Lon	0	rugitt	Lon	0	rtigrit
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane		Yes			Yes			10			10	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	1.00	9	1.00	1.00	9	1.00	1.00	9	1.00	1.00	9
Number of Detectors	1	2	5	1	2	5	1	2	1	10		5
Detector Template	Left	Thru		Left	Thru		Left	Thru	Right			
Leading Detector (ft)	20	100		20	100		20	100	20			
Trailing Detector (ft)	0	0		0	0		0	0	0			
Detector 1 Position(ft)	0	0		0	0		0	0	0			
Detector 1 Size(ft)	20	6		20	6		20	6	20			
Detector 1 Type	CI+Ex	Cl+Ex		CI+Ex	Cl+Ex		CI+Ex	Cl+Ex	CI+Ex			
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0			
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0			
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0			
Detector 2 Position(ft)	0.0	0.0 94		0.0	94		0.0	0.0 94	0.0			
Detector 2 Size(ft)		94			94 6			94 6				
		o Cl+Ex			o Cl+Ex			CI+Ex				
Detector 2 Type Detector 2 Channel												
		0.0			0.0			0.0				
Detector 2 Extend (s)	Dem	0.0		nr 1	0.0		Deres	0.0	Deer			
Turn Type	Perm	NA		pm+pt	NA		Perm	NA	Perm			
Protected Phases		2		1	6			4				

Existing Midday AK

Lanes, Volumes, Timings

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		East Site Driveway & Northern Boulevard (NYS Route 25A)										
	≯	-	$\rightarrow$	-	-	•	1	<b>†</b>	1	1	Ŧ	-
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Permitted Phases	2			6			4		4			
Detector Phase	2	2		1	6		4	4	4			
Switch Phase												
Minimum Initial (s)	10.0	10.0		3.0	10.0		8.0	8.0	8.0			
Minimum Split (s)	18.0	18.0		8.3	18.0		16.0	16.0	16.0			
Total Split (s)	77.0	77.0		19.0	96.0		44.0	44.0	44.0			
Total Split (%)	55.0%	55.0%		13.6%	68.6%		31.4%	31.4%	31.4%			
Maximum Green (s)	69.0	69.0		13.7	88.0		36.0	36.0	36.0			
Yellow Time (s)	5.0	5.0		3.2	5.0		4.0	4.0	4.0			
All-Red Time (s)	3.0	3.0		2.1	3.0		4.0	4.0	4.0			
Lost Time Adjust (s)	0.0	0.0		0.0	0.0			0.0	0.0			
Total Lost Time (s)	8.0	8.0		5.3	8.0			8.0	8.0			
Lead/Lag	Lag	Lag		Lead								
Lead-Lag Optimize?	Yes	Yes		Yes								
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0	3.0			
Recall Mode	C-Max	C-Max		None	C-Max		None	None	None			
Walk Time (s)	7.0	7.0					7.0	7.0	7.0			
Flash Dont Walk (s)	25.0	25.0					22.0	22.0	22.0			
Pedestrian Calls (#/hr)	0	0					0	0	0			
Act Effct Green (s)		113.1		121.0	119.9			8.9	8.9			
Actuated g/C Ratio		0.81		0.86	0.86			0.06	0.06			
v/c Ratio		0.47		0.08	0.37			0.31	0.37			
Control Delay		6.6		0.3	0.3			69.5	14.1			
Queue Delay		0.1		0.0	0.2			0.0	0.1			
Total Delay		6.7		0.3	0.5			69.5	14.1			
LOS		А		Α	А			E	В			
Approach Delay		6.7			0.5			33.1				
Approach LOS		А			Α			С				
Intersection Summary												
Area Type:	Other											
Cycle Length: 140												
Actuated Cycle Length: 140	)											
Offset: 36 (26%), Reference	ed to phase	e 2:EBTL a	and 6:WE	STL, Start	of Yellow							
Natural Cycle: 60												
Control Type: Actuated-Co	ordinated											
Maximum v/c Ratio: 0.47												
Intersection Signal Delay: 4					ntersectior							
Intersection Capacity Utiliza	ation 53.3%	)		10	CU Level o	of Service	Α					
Analysis Period (min) 15												

### Lanes, Volumes, Timings

2: Clapham Avenue/East Site Driveway & Northern Boulevard (NYS Route 25A) Splits and Phases:



	٠	_	+	<b>\</b>	5	7
	EDI			-	-	-
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	<u></u>	<u></u>	- <b>†</b> Þ	<b>6</b> -1	ካካ	1
Traffic Volume (vph)	180	1337	1024	274	470	163
Future Volume (vph)	180	1337	1024	274	470	163
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	110			0	435	115
Storage Lanes	1			0	1	1
Taper Length (ft)	25				25	
Lane Util. Factor	1.00	0.95	0.95	0.95	0.97	1.00
Frt			0.968			0.850
Flt Protected	0.950				0.950	
Satd. Flow (prot)	1752	3505	3393	0	3400	1568
Flt Permitted	0.950				0.950	
Satd. Flow (perm)	1752	3505	3393	0	3400	1568
Right Turn on Red				No		No
Satd. Flow (RTOR)						
Link Speed (mph)		35	35		30	
Link Distance (ft)		300	1113		956	
Travel Time (s)		5.8	21.7		21.7	
Peak Hour Factor	0.00	0.98	0.98	0.00		0.00
	0.98			0.98	0.98	0.98
Heavy Vehicles (%)	3%	3%	3%	3%	3%	3%
Adj. Flow (vph)	184	1364	1045	280	480	166
Shared Lane Traffic (%)		1001	100-			
Lane Group Flow (vph)	184	1364	1325	0	480	166
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right
Median Width(ft)		12	12		24	
Link Offset(ft)		0	0		0	
Crosswalk Width(ft)		16	16		16	
Two way Left Turn Lane		Yes				
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15			9	15	9
Number of Detectors	1	2	2	J	1	1
Detector Template	Left	Thru	Thru		Left	Right
Leading Detector (ft)	20	100	100		20	20
Trailing Detector (ft)	0	0	0		0	0
Detector 1 Position(ft)	0	0	0		0	0
Detector 1 Size(ft)	20	6	6		20	20
Detector 1 Type	CI+Ex	Cl+Ex	Cl+Ex		Cl+Ex	CI+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0		0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0		0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0		0.0	0.0
Detector 2 Position(ft)		94	94			
Detector 2 Size(ft)		6	6			
Detector 2 Type		Cl+Ex	Cl+Ex			
Detector 2 Channel						
Detector 2 Extend (s)		0.0	0.0			
Turn Type	Prot	NA	NA		Prot	Over
Protected Phases		2	6		7	
	5	۷	0		1	5

June 2	2022
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Lane Group	EBL	EBT	WBT	WBR	SBL	SBR	
Permitted Phases							
Detector Phase	5	2	6		7	5	
Switch Phase							
Minimum Initial (s)	8.0	10.0	10.0		8.0	8.0	
Minimum Split (s)	14.5	17.0	36.0		14.5	14.5	
Total Split (s)	27.0	90.0	90.0		33.0	27.0	
Total Split (%)	18.0%	60.0%	60.0%		22.0%	18.0%	
Maximum Green (s)	20.5	83.0	83.0		26.5	20.5	
Yellow Time (s)	4.0	5.0	5.0		4.0	4.0	
All-Red Time (s)	2.5	2.0	2.0		2.5	2.5	
Lost Time Adjust (s)	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.5	7.0	7.0		6.5	6.5	
Lead/Lag							
Lead-Lag Optimize?		~ ~ ~	~ ~		~ ~	<b>6 0</b>	
Vehicle Extension (s)	3.0	3.0	3.0		3.0	3.0	
Recall Mode	None	C-Max	C-Max		None	None	
Walk Time (s)	7.0		7.0		7.0	7.0	
Flash Dont Walk (s)	21.0		22.0		22.0	21.0	
Pedestrian Calls (#/hr)	0	06.0	0		0	0	
Act Effct Green (s)	18.9	86.3	86.3		24.8	18.9	
Actuated g/C Ratio	0.13	0.58	0.58		0.17	0.13	
v/c Ratio	0.83	0.68	0.68		0.86	0.84 96.4	
Control Delay	103.0	19.6 0.4	25.1 0.0		76.1	96.4 0.0	
Queue Delay Total Delay	20.6 123.6	20.0	25.1		0.0 76.1	96.4	
LOS	123.0 F	20.0 B	20.1 C		70.1 E	90.4 F	
Approach Delay	Г	32.3	25.1		81.3	Г	
Approach LOS		52.5 C	23.1 C		61.5 F		
		0	0		1		
Intersection Summary							
Area Type:	Other						
Cycle Length: 150	_						
Actuated Cycle Length: 15							
Offset: 66 (44%), Referen	ced to phase	e 2:EBT a	ind 6:WBT	, Start of	Yellow		
Natural Cycle: 75							
Control Type: Actuated-Co	oordinated						
Maximum v/c Ratio: 0.86	20.0						
Intersection Signal Delay:					ntersection		<b>D</b>
Intersection Capacity Utiliz	zation //.1%	)		IC	U Level	of Service	ט
Analysis Period (min) 15							
Splits and Phases: 1: N	orthern Boul	evard (N	YS Route	25A) & P	landome	Road	
<b>*/*</b> Ø5	- <b>•</b> Ø2 (R)						
	90 s						



Lanes, Volumes, Ti 2: Clapham Avenue	•	Site Dr	iveway	/ & No	rthern	Boule	/ard (N	IYS Ro	oute 25	5A)	Ju	ne 2022
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	۲.	A		۲	A			<del>ا</del>	1			
Traffic Volume (vph)	0	1492	14	16	1171	0	18	Ō	25	0	0	0
Future Volume (vph)	0	1492	14	16	1171	0	18	0	25	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	90		0	130		0	170		0	0		0
Storage Lanes	1		0	1		0	1		1	0		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.999							0.850			
Flt Protected				0.950				0.950				
Satd. Flow (prot)	1845	3501	0	1752	3505	0	0	1752	1568	0	0	0
Flt Permitted				0.132				0.950				
Satd. Flow (perm)	1845	3501	0	243	3505	0	0	1752	1568	0	0	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		1							75			
Link Speed (mph)		35			35			30			30	
Link Distance (ft)		183			300			662			430	
Travel Time (s)		3.6			5.8			15.0			9.8	
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Heavy Vehicles (%)	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%
Adj. Flow (vph)	0	1554	15	17	1220	0	19	0	26	0	0	0
Shared Lane Traffic (%)	•					•		Ţ	-•	· ·	•	
Lane Group Flow (vph)	0	1569	0	17	1220	0	0	19	26	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			0			0	, igni
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane		Yes			Yes						.•	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	1.00	9	15		9	15	1.00	9	15	1.00	9
Number of Detectors	1	2	Ű	1	2	Ŭ	1	2	1	10		Ű
Detector Template	Left	Thru		Left	Thru		Left	Thru	Right			
Leading Detector (ft)	20	100		20	100		20	100	20			
Trailing Detector (ft)	0	0		0	0		0	0	0			
Detector 1 Position(ft)	0	0		0	0 0		Ŭ Û	0	0			
Detector 1 Size(ft)	20	6		20	6		20	6	20			
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		CI+Ex	Cl+Ex	CI+Ex			
Detector 1 Channel	01 2/	01 24		01 24	01 2/		01 2/	01 24	01 2/			
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0			
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0			
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0			
Detector 2 Position(ft)	0.0	94		0.0	94		0.0	94	0.0			
Detector 2 Size(ft)		6			6			6				
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex				
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0				
Turn Type	Perm	NA		pm+pt	NA		Perm	NA	Perm			
Protected Phases		2		μπ+μι 1	6			4				
		2		I	U			4				

Existing PM AK

2: Clapham Avenu	0	Site Dri	veway	/ & No	rthern I	Boule	vard (N	IYS R	oute 25	iΑ)	Jur	ne 2022
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Permitted Phases	2			6			4		4			
Detector Phase	2	2		1	6		4	4	4			
Switch Phase												
Minimum Initial (s)	10.0	10.0		3.0	10.0		8.0	8.0	8.0			
Minimum Split (s)	18.0	18.0		8.3	18.0		16.0	16.0	16.0			
Total Split (s)	94.0	94.0		19.0	113.0		37.0	37.0	37.0			
Total Split (%)	62.7%	62.7%		12.7%	75.3%		24.7%	24.7%	24.7%			
Maximum Green (s)	86.0	86.0		13.7	105.0		29.0	29.0	29.0			
Yellow Time (s)	5.0	5.0		3.2	5.0		4.0	4.0	4.0			
All-Red Time (s)	3.0	3.0		2.1	3.0		4.0	4.0	4.0			
Lost Time Adjust (s)	0.0	0.0		0.0	0.0			0.0	0.0			
Total Lost Time (s)	8.0	8.0		5.3	8.0			8.0	8.0			
Lead/Lag	Lag	Lag		Lead								
Lead-Lag Optimize?	Yes	Yes		Yes								
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0	3.0			
Recall Mode	C-Max	C-Max		None	C-Max		None	None	None			
Walk Time (s)	7.0	7.0					7.0	7.0	7.0			
Flash Dont Walk (s)	25.0	25.0					22.0	22.0	22.0			
Pedestrian Calls (#/hr)	0	0					0	0	0			
Act Effct Green (s)		126.1		131.6	130.5			8.3	8.3			
Actuated g/C Ratio		0.84		0.88	0.87			0.06	0.06			
v/c Ratio		0.53		0.06	0.40			0.20	0.17			
Control Delay		5.8		0.8	1.0			72.5	2.3			
Queue Delay		0.1		0.0	0.3			0.0	0.4			
Total Delay		6.0		0.8	1.4			72.5	2.7			
LOS		А		Α	А			E	А			
Approach Delay		6.0			1.4			32.2				
Approach LOS		А			А			С				
Intersection Summary												
Area Type:	Other											
Cycle Length: 150												
Actuated Cycle Length: 15	0											
Offset: 98 (65%), Reference	ed to phase	e 2:EBTL a	and 6:WE	STL, Starl	of Yellow							
Natural Cycle: 65												
Control Type: Actuated-Co	ordinated											
Maximum v/c Ratio: 0.53												
Intersection Signal Delay: 4				Ir	ntersectior	LOS: A						
Intersection Capacity Utiliz	ation 61.7%	Ď		10	CU Level o	of Service	в					
Analysis Period (min) 15												

### Lanes, Volumes, Timings

#### Splits and Phases: 2: Clapham Avenue/East Site Driveway & Northern Boulevard (NYS Route 25A)



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	-					-
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	<u></u>		<b>↑</b> Ъ	<b>6</b> - <b>6</b>	ካካ	1
Traffic Volume (vph)	191	1194	894	273	352	198
Future Volume (vph)	191	1194	894	273	352	198
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	110			0	435	115
Storage Lanes	1			0	1	1
Taper Length (ft)	25				25	
Lane Util. Factor	1.00	0.95	0.95	0.95	0.97	1.00
Frt			0.965			0.850
Flt Protected	0.950				0.950	
Satd. Flow (prot)	1752	3505	3382	0	3400	1568
Flt Permitted	0.950				0.950	
Satd. Flow (perm)	1752	3505	3382	0	3400	1568
Right Turn on Red				No	0100	No
Satd. Flow (RTOR)				110		
Link Speed (mph)		35	35		30	
Link Distance (ft)		300	1113		956	
		5.8	21.7			
Travel Time (s)	0.04			0.04	21.7	0.04
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94
Heavy Vehicles (%)	3%	3%	3%	3%	3%	3%
Adj. Flow (vph)	203	1270	951	290	374	211
Shared Lane Traffic (%)						
Lane Group Flow (vph)	203	1270	1241	0	374	211
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right
Median Width(ft)		12	12		24	
Link Offset(ft)		0	0		0	
Crosswalk Width(ft)		16	16		16	
Two way Left Turn Lane		Yes				
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15			9	15	9
Number of Detectors	1	2	2	0	1	1
Detector Template	Left	Thru	Thru		Left	Right
Leading Detector (ft)	20	100	100		20	20
Trailing Detector (ft)	0	0	0		0	0
Detector 1 Position(ft)	0	0	0		0	0
Detector 1 Size(ft)	20	6	6		20	20
Detector 1 Type	CI+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0		0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0		0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0		0.0	0.0
Detector 2 Position(ft)		94	94			
Detector 2 Size(ft)		6	6			
Detector 2 Type		Cl+Ex	CI+Ex			
Detector 2 Channel						
Detector 2 Extend (s)		0.0	0.0			
Turn Type	Prot	NA	NA		Prot	Over
Protected Phases		2	6		7	
	5	۷	0		1	5

Existing Saturday AK

June 2	2022
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Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Permitted Phases						
Detector Phase	5	2	6		7	5
Switch Phase						
Minimum Initial (s)	8.0	10.0	10.0		8.0	8.0
Minimum Split (s)	14.5	17.0	36.0		14.5	14.5
Total Split (s)	29.0	82.0	82.0		29.0	29.0
Total Split (%)	20.7%	58.6%	58.6%		20.7%	20.7%
Maximum Green (s)	22.5	75.0	75.0		22.5	22.5
Yellow Time (s)	4.0	5.0	5.0		4.0	4.0
All-Red Time (s)	2.5	2.0	2.0		2.5	2.5
Lost Time Adjust (s)	0.0	0.0	0.0		0.0	0.0
Total Lost Time (s)	6.5	7.0	7.0		6.5	6.5
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0	3.0	3.0		3.0	3.0
Recall Mode	None	C-Max	C-Max		None	None
Walk Time (s)	7.0		7.0		7.0	7.0
Flash Dont Walk (s)	21.0		22.0		22.0	21.0
Pedestrian Calls (#/hr)	0		0		0	0
Act Effct Green (s)	21.7	78.4	78.4		19.9	21.7
Actuated g/C Ratio	0.16	0.56	0.56		0.14	0.16
v/c Ratio	0.75	0.65	0.66		0.77	0.87
Control Delay	75.7	20.1	24.2		69.1	89.3
Queue Delay	13.9	0.3	0.0		0.0	0.0
Total Delay	89.6	20.5	24.2		69.1	89.3
LOS	F	С	С		E	F
Approach Delay		30.0	24.2		76.4	
Approach LOS		С	С		Е	
Intersection Summary						
Area Type:	Other					
Cycle Length: 140						
Actuated Cycle Length: 14						
Offset: 27 (19%), Reference	ced to phase	e 2:EBT a	nd 6:WBT	, Start of	Yellow	
Natural Cycle: 70						
Control Type: Actuated-Co	ordinated					
Maximum v/c Ratio: 0.87						
Intersection Signal Delay:					ntersectio	
Intersection Capacity Utiliz	ation 70.7%	)		IC	CU Level	of Service
Analysis Period (min) 15						
Splits and Phases: 1: N	orthern Boul	evard (N	YS Route	25A) & P	landome	Road
¥ Ø5	- <b>→</b> ø2	(R)				



2: Clapham Avenue/East Site Driveway & Northern Boulevard (NYS Route 25A)       Ju         Image: Constraint of the state o	ne 2022 SBR 0 0 0 1900 0 1.00 0 0 0
Lane Configurations       1       1       1       1       1       1       1       0       11       0       11       0       11       0       0       1374       9       9       1083       0       11       0       11       0       0       11       0       0       11       0       0       11       0       0       11       0       0       11       0       0       11       0       0       11       0       0       11       0       0       11       0       0       11       0       0       0       0       0       0       0       11       0       11       0<	0 0 1900 0 0 1.00
Lane Configurations       1       1       1       1       1       1       1       0       11       0       11       0       0       1374       9       9       1083       0       11       0       11       0       0       11       0       0       11       0       0       11       0       0       11       0       0       11       0       0       11       0       0       11       0       0       11       0       0       11       0       0       11       0       0       11       0       0       0       0       0       0       11       0       11       0<	0 0 1900 0 0 1.00
Traffic Volume (vph)       0       1374       9       9       1083       0       11       0       11       0       0         Future Volume (vph)       0       1374       9       9       1083       0       11       0       11       0       0         Ideal Flow (vph)       1900	0 1900 0 0 1.00
Future Volume (vph)         0         1374         9         9         1083         0         11         0         11         0         0           Ideal Flow (vphpl)         1900 <t< td=""><td>1900 0 0 1.00</td></t<>	1900 0 0 1.00
Ideal Flow (vphpl)         1900 <td>0 0 1.00</td>	0 0 1.00
Storage Length (ft)         90         0         130         0         170         0         0           Storage Lanes         1         0         1         0         1         1         0           Taper Length (ft)         25         25         25         25         25           Lane Util. Factor         1.00         0.95         0.95         1.00         1.00         1.00         1.00         1.00           Frt         0.999         0.950         0.950         0.950         0.950         0.950	0 0 1.00
Storage Lanes         1         0         1         0         1         0           Taper Length (ft)         25         25         25         25         25         25           Lane Util. Factor         1.00         0.95         0.95         1.00         0.95         1.00	1.00
Taper Length (ft)         25         25         25           Lane Util. Factor         1.00         0.95         0.95         1.00	
Lane Util. Factor         1.00         0.95         0.95         1.00         0.95         1.00         0.850         0.850         0.950         0.950         0.950         0.950         0.950         0.950         0.950         0.950         0.950         0.950         0.950         0.950         0.950         0.950	
Frt         0.999         0.850           Flt Protected         0.950         0.950	0
	0
Sate Flow (prot) 1845 3501 0 1752 3505 0 0 1752 1569 0 0	0
Jalu, HUW (PIU) 1040 JUL U 1702 JUD U U 1702 1000 U U	
Flt Permitted 0.148 0.950	
Satd. Flow (perm) 1845 3501 0 273 3505 0 0 1752 1568 0 0	0
Right Turn on Red Yes Yes Yes	Yes
Satd. Flow (RTOR) 1 80	
Link Speed (mph) 35 35 30 30	
Link Distance (ft) 183 300 662 430	
Travel Time (s) 3.6 5.8 15.0 9.8	
Peak Hour Factor 0.93 0.93 0.93 0.93 0.93 0.93 0.93 0.93	0.93
Heavy Vehicles (%) 3% 3% 3% 3% 3% 3% 3% 3% 3% 3% 3% 3%	3%
Adj. Flow (vph) 0 1477 10 10 1165 0 12 0 12 0 0	0
Shared Lane Traffic (%)	
Lane Group Flow (vph) 0 1487 0 10 1165 0 0 12 12 0 0	0
Enter Blocked Intersection No	No
Lane Alignment Left Left Right Left Left Right Left Right Left Left	Right
Median Width(ft) 12 12 0 0	Ŭ
Link Offset(ft) 0 0 0 0	
Crosswalk Width(ft) 16 16 16 16	
Two way Left Turn Lane Yes Yes	
Headway Factor 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.0	1.00
Turning Speed (mph) 15 9 15 9 15 9 15	9
Number of Detectors 1 2 1 2 1 2 1	
Detector Template Left Thru Left Thru Left Thru Right	
Leading Detector (ft) 20 100 20 100 20 100 20	
Trailing Detector (ft) 0 0 0 0 0 0 0	
Detector 1 Position(ft) 0 0 0 0 0 0 0 0	
Detector 1 Size(ft) 20 6 20 6 20 6 20	
Detector 1 Type CI+Ex CI+Ex CI+Ex CI+Ex CI+Ex CI+Ex CI+Ex	
Detector 1 Channel	
Detector 1 Extend (s) 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	
Detector 1 Queue (s) 0.0 0.0 0.0 0.0 0.0 0.0 0.0	
Detector 1 Delay (s) 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	
Detector 2 Position (ft) 94 94 94	
Detector 2 Size(ft) 6 6 6	
Detector 2 Type CI+Ex CI+Ex CI+Ex	
Detector 2 Channel	
Detector 2 Extend (s) 0.0 0.0 0.0	
Turn Type Perm NA pm+pt NA Perm NA Perm	
Protected Phases 2 1 6 4	

Existing Saturday AK

Z. Ciapham Avenu	le/East	/East Site Driveway & Northern Boulevard (NYS Route 25A)								Jui	ne 2022	
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBF
Permitted Phases	2			6			4		4			
Detector Phase	2	2		1	6		4	4	4			
Switch Phase												
Minimum Initial (s)	10.0	10.0		3.0	10.0		8.0	8.0	8.0			
Minimum Split (s)	18.0	18.0		8.3	18.0		16.0	16.0	16.0			
Total Split (s)	77.0	77.0		19.0	96.0		44.0	44.0	44.0			
Total Split (%)	55.0%	55.0%		13.6%	68.6%		31.4%	31.4%	31.4%			
Maximum Green (s)	69.0	69.0		13.7	88.0		36.0	36.0	36.0			
Yellow Time (s)	5.0	5.0		3.2	5.0		4.0	4.0	4.0			
All-Red Time (s)	3.0	3.0		2.1	3.0		4.0	4.0	4.0			
Lost Time Adjust (s)	0.0	0.0		0.0	0.0			0.0	0.0			
Total Lost Time (s)	8.0	8.0		5.3	8.0			8.0	8.0			
Lead/Lag	Lag	Lag		Lead								
Lead-Lag Optimize?	Yes	Yes		Yes								
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0	3.0			
Recall Mode	C-Max	C-Max		None	C-Max		None	None	None			
Walk Time (s)	7.0	7.0					7.0	7.0	7.0			
Flash Dont Walk (s)	25.0	25.0					22.0	22.0	22.0			
Pedestrian Calls (#/hr)	0	0					0	0	0			
Act Effct Green (s)		123.3		125.1	125.6			8.0	8.0			
Actuated g/C Ratio		0.88		0.89	0.90			0.06	0.06			
v/c Ratio		0.48		0.03	0.37			0.12	0.07			
Control Delay		4.1		0.1	0.3			65.5	0.8			
Queue Delay		0.2		0.0	0.2			0.0	0.0			
Total Delay		4.3		0.1	0.4			65.5	0.8			
LOS		А		Α	А			E	Α			
Approach Delay		4.3			0.4			33.1				
Approach LOS		А			Α			С				
Intersection Summary												
Area Type:	Other											
Cycle Length: 140												
Actuated Cycle Length: 14												
Offset: 36 (26%), Reference	ed to phase	e 2:EBTL a	and 6:WB	STL, Start	of Yellow							
Natural Cycle: 60												
Control Type: Actuated-Co	ordinated											
Maximum v/c Ratio: 0.48												
Intersection Signal Delay: 2					ntersection							
Intersection Capacity Utiliz	ation 58.3%	)		10	CU Level o	of Service	B					
Analysis Period (min) 15												

2: Clapham Avenue/East Site Driveway & Northern Boulevard (NYS Route 25A) Splits and Phases:



Lane Group         EBL         EBT         WBT         WBR         SBL         SBR           Lane Configurations         1
Lane Configurations         Image: Configuration in the image: Configuratine image: Configuration in the image: Configuratine image: Confi
Lane Configurations         Image: Configuration in the image: Configuratine image: Configuration in the image: Configuratine image: Confi
Traffic Volume (vph)         152         832         971         404         494         163           Future Volume (vph)         152         832         971         404         494         163           Ideal Flow (vphpl)         1900         1900         1900         1900         1900         1900         1900           Storage Length (ft)         110         0         435         115           Storage Lanes         1         0         1         1           Taper Length (ft)         25         25         25           Lane Util. Factor         1.00         0.95         0.95         0.97         1.00           Frt         0.956         0.950         0.950         0.850         0.850           Fit Protected         0.950         0.950         0.950         0.850           Satd. Flow (prot)         1752         3505         3351         0         3400         1568           Fit Permitted         0.950         0.950         0.950         Satd. Flow (perm)         1752         3505         3351         0         3400         1568           Right Turn on Red         No         No         No         Satd. Flow (RTOR)         1113
Future Volume (vph)         152         832         971         404         494         163           Ideal Flow (vphpl)         1900         1900         1900         1900         1900         1900         1900           Storage Length (ft)         110         0         435         115           Storage Lanes         1         0         1         1           Taper Length (ft)         25         25         25           Lane Util. Factor         1.00         0.95         0.95         0.97         1.00           Frt         0.956         0.950         0.950         0.850         0.850         0.850           Fit Protected         0.950         3351         0         3400         1568         0.950         Satd. Flow (prot)         1752         3505         3351         0         3400         1568         8         Right Turn on Red         No         No         No         Satd. Flow (perm)         1752         3505         3351         0         3400         1568         8         Right Turn on Red         No         No         Satd. Flow (RTOR)         No         No         Satd. Flow (RTOR)         Satd. Flow (RTOR)         Satd. Flow (RTOR)         Satd. Flow (RTOR) <td< td=""></td<>
Ideal Flow (vphpl)         1900         110         100         111         100         110         100         110         100         100         110         100         100         110         100         100         110         110         100         110         110         110 <th110< th="">         11568         1110</th110<>
Storage Length (ft)       110       0       435       115         Storage Lanes       1       0       1       1         Taper Length (ft)       25       25         Lane Util. Factor       1.00       0.95       0.95       0.97       1.00         Frt       0.956       0.950       0.850       0.850         Flt Protected       0.950       0.950       0.950       0.950         Satd. Flow (prot)       1752       3505       3351       0       3400       1568         Flt Permitted       0.950       0.950       0.950       0.950       0.950       0.950         Satd. Flow (perm)       1752       3505       3351       0       3400       1568         Right Turn on Red       No       No       No       No       Satd. Flow (RTOR)       No         Link Speed (mph)       35       35       30       1113       956         Travel Time (s)       5.8       21.7       21.7       21.7
Storage Lanes       1       0       1       1         Taper Length (ft)       25       25       25         Lane Util. Factor       1.00       0.95       0.95       0.97       1.00         Frt       0.956       0.950       0.850       0.850         Flt Protected       0.950       0.950       0.950       0.950         Satd. Flow (prot)       1752       3505       3351       0       3400       1568         Flt Permitted       0.950       0.950       0.950       0.950       0.950       0.950         Satd. Flow (perm)       1752       3505       3351       0       3400       1568         Right Turn on Red       No       No       No       No       Satd. Flow (RTOR)       No         Link Speed (mph)       35       35       30       1113       956         Travel Time (s)       5.8       21.7       21.7       21.7
Taper Length (ft)         25         25           Lane Util. Factor         1.00         0.95         0.95         0.97         1.00           Frt         0.956         0.950         0.850           Filt Protected         0.950         3351         0         3400         1568           Filt Permitted         0.950         0.950         0.950         0.950           Satd. Flow (prot)         1752         3505         3351         0         3400         1568           Filt Permitted         0.950         0.950         0.950         0.950         0.950           Satd. Flow (perm)         1752         3505         3351         0         3400         1568           Right Turn on Red         No         No         No         No         Satd. Flow (RTOR)         1113         956           Link Distance (ft)         300         1113         956         1113         956
Lane Util. Factor         1.00         0.95         0.95         0.95         0.97         1.00           Frt         0.956         0.956         0.850         0.850         0.850         0.850         0.850         0.950 <td< td=""></td<>
Frt         0.956         0.850           Fit Protected         0.950         0.950           Satd. Flow (prot)         1752         3505         3351         0         3400         1568           Fit Permitted         0.950         0.950         0.950         0.950           Satd. Flow (perm)         1752         3505         3351         0         3400         1568           Right Turn on Red         No         No         No         No           Satd. Flow (RTOR)         35         35         30           Link Speed (mph)         35         35         30           Link Distance (ft)         300         1113         956           Travel Time (s)         5.8         21.7         21.7
Fit Protected         0.950         0.950           Satd. Flow (prot)         1752         3505         3351         0         3400         1568           Fit Permitted         0.950         0.950         0.950         0.950           Satd. Flow (perm)         1752         3505         3351         0         3400         1568           Right Turn on Red         No         No         No         No           Satd. Flow (RTOR)         35         35         30           Link Speed (mph)         35         35         30           Link Distance (ft)         300         1113         956           Travel Time (s)         5.8         21.7         21.7
Satd. Flow (prot)         1752         3505         3351         0         3400         1568           Flt Permitted         0.950
Fit Permitted         0.950         0.950           Satd. Flow (perm)         1752         3505         3351         0         3400         1568           Right Turn on Red         No         No         No           Satd. Flow (RTOR)         35         35         30           Link Speed (mph)         35         35         30           Link Distance (ft)         300         1113         956           Travel Time (s)         5.8         21.7         21.7
Satd. Flow (perm)         1752         3505         3351         0         3400         1568           Right Turn on Red         No         No         No         Satd.         Satd. Flow (RTOR)         Isometry         Isometry         Isometry         Satd.         Satd. <td< td=""></td<>
Right Turn on Red         No         No           Satd. Flow (RTOR)         35         35         30           Link Speed (mph)         35         35         30           Link Distance (ft)         300         1113         956           Travel Time (s)         5.8         21.7         21.7
Satd. Flow (RTOR)           Link Speed (mph)         35         35         30           Link Distance (ft)         300         1113         956           Travel Time (s)         5.8         21.7         21.7
Link Speed (mph)         35         35         30           Link Distance (ft)         300         1113         956           Travel Time (s)         5.8         21.7         21.7
Link Distance (ft)         300         1113         956           Travel Time (s)         5.8         21.7         21.7
Travel Time (s) 5.8 21.7 21.7
Travel Time (s) 5.8 21.7 21.7
Heavy Vehicles (%) 3% 3% 3% 3% 3% 3%
Adj. Flow (vph) 157 858 1001 416 509 168
Shared Lane Traffic (%)
Lane Group Flow (vph) 157 858 1417 0 509 168
Enter Blocked Intersection No No No No No No
Link Offset(ft) 0 0 0
Crosswalk Width(ft) 16 16 16
Two way Left Turn Lane Yes
Headway Factor 1.00 1.00 1.00 1.00 1.00 1.00
Turning Speed (mph)         15         9         15         9
Number of Detectors 1 2 2 1 1
Detector Template Left Thru Thru Left Right
Leading Detector (ft) 20 100 100 20 20
Trailing Detector (ft) 0 0 0 0 0
Detector 1 Position(ft) 0 0 0 0 0
Detector 1 Size(ft) 20 6 6 20 20
Detector 1 Type CI+Ex CI+Ex CI+Ex CI+Ex CI+Ex
Detector 1 Channel
Detector 1 Extend (s) 0.0 0.0 0.0 0.0 0.0
Detector 1 Queue (s) 0.0 0.0 0.0 0.0 0.0 0.0 0.0
<b>3</b> ( )
Detector 2 Position(ft) 94 94
Detector 2 Size(ft) 6 6
Detector 2 Type CI+Ex CI+Ex
Detector 2 Channel
Detector 2 Extend (s) 0.0 0.0
Turn Type Prot NA NA Prot Over
Protected Phases 5 2 6 7 5

June 2	2022
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Lane Group	EBL	EBT	WBT	WBR	SBL	SBR	
Permitted Phases							
Detector Phase	5	2	6		7	5	
Switch Phase							
Minimum Initial (s)	8.0	10.0	10.0		8.0	8.0	
Minimum Split (s)	14.5	17.0	36.0		14.5	14.5	
Total Split (s)	27.0	90.0	90.0		33.0	27.0	
Total Split (%)	18.0%	60.0%	60.0%		22.0%	18.0%	
Maximum Green (s)	20.5	83.0	83.0		26.5	20.5	
Yellow Time (s)	4.0	5.0	5.0		4.0	4.0	
All-Red Time (s)	2.5	2.0	2.0		2.5	2.5	
Lost Time Adjust (s)	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.5	7.0	7.0		6.5	6.5	
Lead/Lag							
Lead-Lag Optimize?							
Vehicle Extension (s)	3.0	3.0	3.0		3.0	3.0	
Recall Mode	None	C-Max	C-Max		None	None	
Walk Time (s)	7.0		7.0		7.0	7.0	
Flash Dont Walk (s)	21.0		22.0		22.0	21.0	
Pedestrian Calls (#/hr)	0		0		0	0	
Act Effct Green (s)	19.0	85.6	85.6		25.4	19.0	
Actuated g/C Ratio	0.13	0.57	0.57		0.17	0.13	
v/c Ratio	0.71	0.43	0.74		0.89	0.85	
Control Delay	97.3	14.8	27.5		78.6	97.4	
Queue Delay	6.9	0.4	0.0		0.0	0.0	
Total Delay	104.2	15.2	27.5		78.6	97.4	
LOS	F	В	С		E	F	
Approach Delay		29.0	27.5		83.2		
Approach LOS		С	С		F		
Intersection Summary							
Area Type:	Other						
Cycle Length: 150							
Actuated Cycle Length: 1	50						
Offset: 14 (9%), Referend		2.FRT an	d 6·WRT	Start of	fellow		
Natural Cycle: 90			u 0.001,		GIOW		
Control Type: Actuated-C	Coordinated						
Maximum v/c Ratio: 0.89							
Intersection Signal Delay				Ir	ntersectio	n   09· D	
Intersection Capacity Util						of Service	
Analysis Period (min) 15	2au01170.3/0	J		K			, 0
	Northern Bou	levard (N	YS Route	25A) & P	landome	Road	
<b>₽</b> Ø5	→ø2 (R)						
27 s	90 s						

Ø5	→Ø2 (R)	<b>Ø</b> 7
27 s	90 s	33 s
	+	
	Ø6 (R)	
	90 s	

Lanes, Volumes, Ti 2: Clapham Avenue	•	Sito Dr		( 8. No	rthorn	Poulo	ord (N		outo 25		hu	ne 2022
					<b>4</b>			<b>†</b>		<u>,                                    </u>	Ļ	IC 2022
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	<b>≜</b> †⊅		5	<b>≜</b> †⊅			र्भ	1			
Traffic Volume (vph)	0	927	10	17	1117	0	39	Ō	57	0	0	0
Future Volume (vph)	0	927	10	17	1117	0	39	0	57	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	90		0	130		0	170		0	0		0
Storage Lanes	1		0	1		0	1		1	0		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.998							0.850			
Flt Protected		0.000		0.950				0.950	0.000			
Satd. Flow (prot)	1845	3498	0	1752	3505	0	0	1752	1568	0	0	0
Flt Permitted	1010	0100	Ű	0.259	0000	Ŭ	Ŭ	0.950	1000	Ū	Ŭ	Ū
Satd. Flow (perm)	1845	3498	0	478	3505	0	0	1752	1568	0	0	0
Right Turn on Red	10-10	0400	Yes	10	0000	Yes	U	1102	Yes	U	U	Yes
Satd. Flow (RTOR)		1	163			163			75			163
Link Speed (mph)		35			35			30	15		30	
Link Distance (ft)		183			300			662			430	
		3.6			5.8			15.0			430 9.8	
Travel Time (s)	0.02		0.02	0.02		0.02	0.02		0.02	0.02		0.02
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%
Adj. Flow (vph)	0	997	11	18	1201	0	42	0	61	0	0	0
Shared Lane Traffic (%)	0	4000	0	40	4004	0	0	10	04	0	0	
Lane Group Flow (vph)	0	1008	0	18	1201	0	0	42	61	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane		Yes			Yes							
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2		1	2		1	2	1			
Detector Template	Left	Thru		Left	Thru		Left	Thru	Right			
Leading Detector (ft)	20	100		20	100		20	100	20			
Trailing Detector (ft)	0	0		0	0		0	0	0			
Detector 1 Position(ft)	0	0		0	0		0	0	0			
Detector 1 Size(ft)	20	6		20	6		20	6	20			
Detector 1 Type	Cl+Ex	CI+Ex		Cl+Ex	CI+Ex		CI+Ex	Cl+Ex	CI+Ex			
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0			
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0			
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0			
Detector 2 Position(ft)		94			94			94				
Detector 2 Size(ft)		6			6			6				
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex				
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0				
Turn Type	Perm	NA		pm+pt	NA		Perm	NA	Perm			
Protected Phases		2		1	6			4				
		4			v			7				

No Build AM AK

2: Clapham Avenue/East Site Driveway & Northern Boulevard (NYS Route 25A) June 20									ne 2022			
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Permitted Phases	2			6			4		4			
Detector Phase	2	2		1	6		4	4	4			
Switch Phase												
Minimum Initial (s)	10.0	10.0		3.0	10.0		8.0	8.0	8.0			
Minimum Split (s)	18.0	18.0		8.3	18.0		16.0	16.0	16.0			
Total Split (s)	95.0	95.0		18.0	113.0		37.0	37.0	37.0			
Total Split (%)	63.3%	63.3%		12.0%	75.3%		24.7%	24.7%	24.7%			
Maximum Green (s)	87.0	87.0		12.7	105.0		29.0	29.0	29.0			
Yellow Time (s)	5.0	5.0		3.2	5.0		4.0	4.0	4.0			
All-Red Time (s)	3.0	3.0		2.1	3.0		4.0	4.0	4.0			
Lost Time Adjust (s)	0.0	0.0		0.0	0.0			0.0	0.0			
Total Lost Time (s)	8.0	8.0		5.3	8.0			8.0	8.0			
Lead/Lag	Lag	Lag		Lead								
Lead-Lag Optimize?	Yes	Yes		Yes								
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0	3.0			
Recall Mode	C-Max	C-Max		None	C-Max		None	None	None			
Walk Time (s)	7.0	7.0					7.0	7.0	7.0			
Flash Dont Walk (s)	25.0	25.0					22.0	22.0	22.0			
Pedestrian Calls (#/hr)	0	0					0	0	0			
Act Effct Green (s)		122.5		130.3	129.2			9.6	9.6			
Actuated g/C Ratio		0.82		0.87	0.86			0.06	0.06			
v/c Ratio		0.35		0.04	0.40			0.38	0.36			
Control Delay		5.4		0.4	0.6			76.4	14.5			
Queue Delay		0.0		0.0	0.4			0.0	0.1			
Total Delay		5.5		0.4	1.0			76.4	14.6			
LOS		А		А	А			E	В			
Approach Delay		5.5			1.0			39.8				
Approach LOS		А			А			D				
Intersection Summary												
Area Type:	Other											
Cycle Length: 150												
Actuated Cycle Length: 150	0											
Offset: 39 (26%), Referenc	ed to phase	e 2:EBTL a	and 6:WE	BTL, Starl	t of Yellow							
Natural Cycle: 50												
Control Type: Actuated-Co	ordinated											
Maximum v/c Ratio: 0.40						100.1						
Intersection Signal Delay: 4		,			ntersectior							
Intersection Capacity Utiliza	ation 50.9%	)		10	CU Level o	of Service	A					
Analysis Period (min) 15												

## Splits and Phases: 2: Clapham Avenue/East Site Driveway & Northern Boulevard (NYS Route 25A)



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		EDT		\ <u>\</u> /DD	CDL	000
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	<b>1</b>		<b>††</b>	070	ካካ	7
Traffic Volume (vph)	166	1226	991	270	504	175
Future Volume (vph)	166	1226	991	270	504	175
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	110			0	435	115
Storage Lanes	1			0	1	1
Taper Length (ft)	25				25	
Lane Util. Factor	1.00	0.95	0.95	0.95	0.97	1.00
Frt			0.968			0.850
Flt Protected	0.950				0.950	
Satd. Flow (prot)	1752	3505	3393	0	3400	1568
Flt Permitted	0.950	2000		Ű	0.950	
Satd. Flow (perm)	1752	3505	3393	0	3400	1568
Right Turn on Red	11.52	0000	0000	No	5-00	No
Satd. Flow (RTOR)				NU		NU
		35	35		30	
Link Speed (mph)						
Link Distance (ft)		300	1113		956	
Travel Time (s)	0.00	5.8	21.7	0.00	21.7	0.00
Peak Hour Factor	0.89	0.89	0.89	0.89	0.89	0.89
Heavy Vehicles (%)	3%	3%	3%	3%	3%	3%
Adj. Flow (vph)	187	1378	1113	303	566	197
Shared Lane Traffic (%)						
Lane Group Flow (vph)	187	1378	1416	0	566	197
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right
Median Width(ft)		12	12	Ŭ	24	Ŭ
Link Offset(ft)		0	0		0	
Crosswalk Width(ft)		16	16		16	
Two way Left Turn Lane		Yes				
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	1.00	1.00	1.00	9	1.00	9
Number of Detectors	15	2	2	3	15	9
	Left		∠ Thru		•	
Detector Template		Thru			Left	Right
Leading Detector (ft)	20	100	100		20	20
Trailing Detector (ft)	0	0	0		0	0
Detector 1 Position(ft)	0	0	0		0	0
Detector 1 Size(ft)	20	6	6		20	20
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	CI+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0		0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0		0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0		0.0	0.0
Detector 2 Position(ft)		94	94			
Detector 2 Size(ft)		6	6			
Detector 2 Type		CI+Ex	Cl+Ex			
Detector 2 Channel						
Detector 2 Extend (s)		0.0	0.0			
	D+				Drat	Our
Turn Type	Prot	NA	NA		Prot	Over
Protected Phases	5	2	6		7	5

No Build Midday AK

June 2	2022
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Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Permitted Phases						
Detector Phase	5	2	6		7	5
Switch Phase						
Minimum Initial (s)	8.0	10.0	10.0		8.0	8.0
Minimum Split (s)	14.5	17.0	36.0		14.5	14.5
Total Split (s)	29.0	82.0	82.0		29.0	29.0
Total Split (%)	20.7%	58.6%	58.6%		20.7%	20.7%
Maximum Green (s)	22.5	75.0	75.0		22.5	22.5
Yellow Time (s)	4.0	5.0	5.0		4.0	4.0
All-Red Time (s)	2.5	2.0	2.0		2.5	2.5
Lost Time Adjust (s)	0.0	0.0	0.0		0.0	0.0
Total Lost Time (s)	6.5	7.0	7.0		6.5	6.5
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0	3.0	3.0		3.0	3.0
Recall Mode	None	C-Max	C-Max		None	None
Walk Time (s)	7.0		7.0		7.0	7.0
Flash Dont Walk (s)	21.0		22.0		22.0	21.0
Pedestrian Calls (#/hr)	0		0		0	0
Act Effct Green (s)	20.7	76.8	76.8		22.5	20.7
Actuated g/C Ratio	0.15	0.55	0.55		0.16	0.15
v/c Ratio	0.72	0.72	0.76		1.04	0.85
Control Delay	84.9	20.2	28.3		104.6	88.5
Queue Delay	7.2	0.4	0.0		0.0	0.0
Total Delay	92.1	20.7	28.3		104.6	88.5
LOS	F	С	С		F	F
Approach Delay		29.2	28.3		100.4	
Approach LOS		С	С		F	
Intersection Summary						
Area Type:	Other					
Cycle Length: 140						
Actuated Cycle Length: 14						
Offset: 27 (19%), Reference	ced to phase	e 2:EBT a	nd 6:WBT	, Start of	Yellow	
Natural Cycle: 90						
Control Type: Actuated-Co	oordinated					
Maximum v/c Ratio: 1.04						
Intersection Signal Delay:						n LOS: D
Intersection Capacity Utiliz	zation 76.3%	)		IC	CU Level	of Service
Analysis Period (min) 15						
Splits and Phases: 1: No	orthern Boul	evard (N	YS Route	25A) & P	landome	Road
				_0, , 0 1		
× 05		(R)				



Lane Group       EBL       EBT       EBR       WBL       WBT       WBR       NBL       NBT       NBR       Si         Lane Configurations       1	0 0 0 0	SBR
Lane Configurations         1	0 0 0 0	
Traffic Volume (vph)         0         1332         27         26         1141         0         32         0         60           Future Volume (vph)         0         1332         27         26         1141         0         32         0         60           Ideal Flow (vphpl)         1900	0 0	
Traffic Volume (vph)         0         1332         27         26         1141         0         32         0         60           Future Volume (vph)         0         1332         27         26         1141         0         32         0         60           Ideal Flow (vphpl)         1900	0 0	
Future Volume (vph)         0         1332         27         26         1141         0         32         0         60           Ideal Flow (vphpl)         1900		0
Ideal Flow (vphpl)         1900 <td>1000</td> <td>0</td>	1000	0
Storage Lanes 1 0 1 0 1 1	0 1900	1900
Storage Lanes 1 0 1 0 1 1	0	0
Tanari anath (ft) 05 05 05	0	0
Taper Length (ft) 25 25 25	5	
Lane Util. Factor 1.00 0.95 0.95 1.00 0.95 0.95 1.00 1.00 1.00 1.	0 1.00	1.00
Frt 0.997 0.850		
Fit Protected 0.950 0.950		
Satd. Flow (prot) 1845 3494 0 1752 3505 0 0 1752 1568	0 0	0
Flt Permitted 0.140 0.950		
Satd. Flow (perm) 1845 3494 0 258 3505 0 0 1752 1568	0 0	0
Right Turn on Red Yes Yes Yes Yes		Yes
Satd. Flow (RTOR) 2 80		
Link Speed (mph) 35 35 30	30	
Link Distance (ft) 183 300 662	430	
Travel Time (s) 3.6 5.8 15.0	9.8	
Peak Hour Factor 0.91 0.91 0.91 0.91 0.91 0.91 0.91 0.91		0.91
	% 3%	3%
Adj. Flow (vph) 0 1464 30 29 1254 0 35 0 66	0 0	0
Shared Lane Traffic (%)		-
Lane Group Flow (vph) 0 1494 0 29 1254 0 0 35 66	0 0	0
	o No	No
	ft Left	Right
Median Width(ft) 12 12 0	0	Ŭ
Link Offset(ft) 0 0 0	0	
Crosswalk Width(ft) 16 16 16	16	
Two way Left Turn Lane Yes Yes		
Headway Factor 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.0	0 1.00	1.00
	5	9
Number of Detectors 1 2 1 2 1 2 1		
Detector Template Left Thru Left Thru Left Thru Right		
Leading Detector (ft) 20 100 20 100 20 100 20		
Trailing Detector (ft) 0 0 0 0 0 0 0		
Detector 1 Position(ft) 0 0 0 0 0 0 0		
Detector 1 Size(ft) 20 6 20 6 20 6 20		
Detector 1 Type CI+Ex CI+Ex CI+Ex CI+Ex CI+Ex CI+Ex CI+Ex		
Detector 1 Channel		
Detector 1 Extend (s) 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0		
Detector 1 Queue (s) 0.0 0.0 0.0 0.0 0.0 0.0 0.0		
Detector 1 Delay (s) 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0		
Detector 2 Position(ft) 94 94 94		
Detector 2 Size(ft) 6 6 6		
Detector 2 Type CI+Ex CI+Ex CI+Ex		
Detector 2 Channel		
Detector 2 Extend (s) 0.0 0.0 0.0		
Turn Type Perm NA pm+pt NA Perm NA Perm		
Protected Phases 2 1 6 4		

No Build Midday AK

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# Lanes, Volumes, Timings

2: Clapham Avenu	_						· · · ·					
	≯	-	$\rightarrow$	-	-	•	1	<b>†</b>	1	-	Ŧ	-
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBF
Permitted Phases	2			6			4		4			
Detector Phase	2	2		1	6		4	4	4			
Switch Phase												
Minimum Initial (s)	10.0	10.0		3.0	10.0		8.0	8.0	8.0			
Minimum Split (s)	18.0	18.0		8.3	18.0		16.0	16.0	16.0			
Total Split (s)	77.0	77.0		19.0	96.0		44.0	44.0	44.0			
Total Split (%)	55.0%	55.0%		13.6%	68.6%		31.4%	31.4%	31.4%			
Maximum Green (s)	69.0	69.0		13.7	88.0		36.0	36.0	36.0			
Yellow Time (s)	5.0	5.0		3.2	5.0		4.0	4.0	4.0			
All-Red Time (s)	3.0	3.0		2.1	3.0		4.0	4.0	4.0			
Lost Time Adjust (s)	0.0	0.0		0.0	0.0			0.0	0.0			
Total Lost Time (s)	8.0	8.0		5.3	8.0			8.0	8.0			
Lead/Lag	Lag	Lag		Lead								
Lead-Lag Optimize?	Yes	Yes		Yes								
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0	3.0			
Recall Mode	C-Max	C-Max		None	C-Max		None	None	None			
Walk Time (s)	7.0	7.0					7.0	7.0	7.0			
Flash Dont Walk (s)	25.0	25.0					22.0	22.0	22.0			
Pedestrian Calls (#/hr)	0	0					0	0	0			
Act Effct Green (s)		113.0		120.9	119.8			9.0	9.0			
Actuated g/C Ratio		0.81		0.86	0.86			0.06	0.06			
v/c Ratio		0.53		0.10	0.42			0.31	0.38			
Control Delay		7.3		0.5	0.3			69.6	14.8			
Queue Delay		0.2		0.0	0.3			0.0	0.1			
Total Delay		7.5		0.5	0.6			69.6	14.8			
LOS		А		Α	А			E	В			
Approach Delay		7.5			0.6			33.8				
Approach LOS		А			Α			С				
Intersection Summary												
Area Type:	Other											
Cycle Length: 140												
Actuated Cycle Length: 14												
Offset: 36 (26%), Reference	ed to phase	e 2:EBTL a	and 6:WE	STL, Starl	t of Yellow							
Natural Cycle: 60												
Control Type: Actuated-Co	ordinated											
Maximum v/c Ratio: 0.53	_											
Intersection Signal Delay:					ntersectior							
Intersection Capacity Utiliz	ation 57.7%	)		10	CU Level o	of Service	Β					
Analysis Period (min) 15												

2: Clapham Avenue/East Site Driveway & Northern Boulevard (NYS Route 25A) Splits and Phases:



	•	_	+		5	7
	-			-		
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		<u></u>	- <b>†</b> Þ		ካካ	1
Traffic Volume (vph)	184	1459	1224	329	504	166
Future Volume (vph)	184	1459	1224	329	504	166
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	110			0	435	115
Storage Lanes	1			0	1	1
Taper Length (ft)	25				25	
Lane Util. Factor	1.00	0.95	0.95	0.95	0.97	1.00
Frt			0.968			0.850
Flt Protected	0.950		0.000		0.950	0.000
Satd. Flow (prot)	1752	3505	3393	0	3400	1568
Flt Permitted	0.950	5505	0000	0	0.950	1500
Satd. Flow (perm)	1752	3505	3393	0	3400	1568
	1752	3000	2292		5400	
Right Turn on Red				No		No
Satd. Flow (RTOR)			<b>^</b> -			
Link Speed (mph)		35	35		30	
Link Distance (ft)		300	1113		956	
Travel Time (s)		5.8	21.7		21.7	
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98
Heavy Vehicles (%)	3%	3%	3%	3%	3%	3%
Adj. Flow (vph)	188	1489	1249	336	514	169
Shared Lane Traffic (%)						
Lane Group Flow (vph)	188	1489	1585	0	514	169
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right
Median Width(ft)	Lon	12	12	ragine	24	rugitt
Link Offset(ft)		0	0		0	
Crosswalk Width(ft)		16	16		16	
( )			10		10	
Two way Left Turn Lane	1 00	Yes	1 00	1 00	4 00	4 00
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	_		9	15	9
Number of Detectors	1	2	2		1	1
Detector Template	Left	Thru	Thru		Left	Right
Leading Detector (ft)	20	100	100		20	20
Trailing Detector (ft)	0	0	0		0	0
Detector 1 Position(ft)	0	0	0		0	0
Detector 1 Size(ft)	20	6	6		20	20
Detector 1 Type	CI+Ex	Cl+Ex	CI+Ex		Cl+Ex	CI+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0		0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0		0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0		0.0	0.0
• • • •	0.0	94	94		0.0	0.0
Detector 2 Position(ft)						
Detector 2 Size(ft)		6 01-5-	6			
Detector 2 Type		Cl+Ex	Cl+Ex			
Detector 2 Channel						
Detector 2 Extend (s)		0.0	0.0			
Turn Type	Prot	NA	NA		Prot	Over
Protected Phases	5	2	6		7	5

June 2	2022
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Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Permitted Phases						
Detector Phase	5	2	6		7	5
Switch Phase						
Minimum Initial (s)	8.0	10.0	10.0		8.0	8.0
Minimum Split (s)	14.5	17.0	36.0		14.5	14.5
Total Split (s)	27.0	90.0	90.0		33.0	27.0
Total Split (%)	18.0%	60.0%	60.0%		22.0%	18.0%
Maximum Green (s)	20.5	83.0	83.0		26.5	20.5
Yellow Time (s)	4.0	5.0	5.0		4.0	4.0
All-Red Time (s)	2.5	2.0	2.0		2.5	2.5
Lost Time Adjust (s)	0.0	0.0	0.0		0.0	0.0
Total Lost Time (s)	6.5	7.0	7.0		6.5	6.5
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0	3.0	3.0		3.0	3.0
Recall Mode	None	C-Max	C-Max		None	None
Walk Time (s)	7.0		7.0		7.0	7.0
Flash Dont Walk (s)	21.0		22.0		22.0	21.0
Pedestrian Calls (#/hr)	0		0		0	0
Act Effct Green (s)	19.1	85.4	85.4		25.5	19.1
Actuated g/C Ratio	0.13	0.57	0.57		0.17	0.13
v/c Ratio	0.85	0.75	0.82		0.89	0.85
Control Delay	103.2	21.8	31.2		79.1	97.5
Queue Delay	23.1	0.7	0.0		0.0	0.0
Total Delay	126.3	22.4	31.2		79.1	97.5
LOS	F	С	С		E	F
Approach Delay		34.1	31.2		83.6	
Approach LOS		C	C		F	
		-	-		-	
Intersection Summary						
Area Type:	Other					
Cycle Length: 150						
Actuated Cycle Length: 15						
Offset: 66 (44%), Referen	ced to phase	e 2:EBT a	nd 6:WBT	, Start of	Yellow	
Natural Cycle: 90						
Control Type: Actuated-Co	oordinated					
Maximum v/c Ratio: 0.89						
Intersection Signal Delay:					ntersectio	
Intersection Capacity Utiliz	zation 85.6%	)		IC	CU Level	of Service
Analysis Period (min) 15						
Splits and Phases: 1: N	orthern Boul	evard (N	YS Route	25A) & P	landome	Road
<b>*</b>						
Ø5 27 s	→Ø2 (R) 90 s					

Ø5	Ø2 (R)	<sup>•</sup> Ø7	
27 s	90 s	33 s	
	+		
	Ø6 (R)		
	90 s		

2: Clapham Avenue							```				_	
	≯	-	$\rightarrow$	4	-	*	1	T.	1	1	Ŧ	-
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	<b>≜1</b> }-		ሻ	<b>∱</b> }			र्च	1			
Traffic Volume (vph)	0	1617	14	16	1374	0	18	0	26	0	0	0
Future Volume (vph)	0	1617	14	16	1374	0	18	0	26	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	90		0	130		0	170		0	0		0
Storage Lanes	1		0	1		0	1		1	0		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.999							0.850			
Flt Protected				0.950				0.950				
Satd. Flow (prot)	1845	3501	0	1752	3505	0	0	1752	1568	0	0	0
Flt Permitted				0.111				0.950				
Satd. Flow (perm)	1845	3501	0	205	3505	0	0	1752	1568	0	0	0
Right Turn on Red			Yes			Yes	•		Yes	· ·	•	Yes
Satd. Flow (RTOR)		1	100						75			
Link Speed (mph)		35			35			30	10		30	
Link Distance (ft)		183			300			662			430	
Travel Time (s)		3.6			5.8			15.0			9.8	
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Heavy Vehicles (%)	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%
Adj. Flow (vph)	0	1684	15	17	1431	0	19	0	27	0	0	0
Shared Lane Traffic (%)	0	1004	15	17	1451	0	19	0	21	0	0	0
Lane Group Flow (vph)	0	1699	0	17	1431	0	0	19	27	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left		Left	Left		Left	Left		Left	Left	
Median Width(ft)	Leit	12	Right	Leit	12	Right	Leit	Leit 0	Right	Leit	Leit 0	Right
Link Offset(ft)		0			0			0			0	
( )		16			16			16			16	
Crosswalk Width(ft)								10			10	
Two way Left Turn Lane	1 00	Yes	1 00	1 00	Yes	1 00	1 00	1 00	1 00	1 00	1 00	1 00
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	0	9	15	0	9	15	0	9	15		9
Number of Detectors	1	2		1	2		1	2	1			
Detector Template	Left	Thru		Left	Thru		Left	Thru	Right			
Leading Detector (ft)	20	100		20	100		20	100	20			
Trailing Detector (ft)	0	0		0	0		0	0	0			
Detector 1 Position(ft)	0	0		0	0		0	0	0			
Detector 1 Size(ft)	20	6		20	6		20	6	20			
Detector 1 Type	CI+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	CI+Ex	CI+Ex			
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0			
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0			
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0			
Detector 2 Position(ft)		94			94			94				
Detector 2 Size(ft)		6			6			6				
Detector 2 Type		CI+Ex			CI+Ex			CI+Ex				
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0				
Turn Type	Perm	NA		pm+pt	NA		Perm	NA	Perm			
Protected Phases		2		1	6			4				

No Build PM AK

2: Clapham Avenu	•	Site Dri	veway	/ & No	rthern I	Boulev	vard (N	IYS R	oute 25	A)	Jur	ne 2022
	٦	-	$\mathbf{F}$	4	←	•	1	Ť	۲	1	Ļ	~
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Permitted Phases	2			6			4		4			
Detector Phase	2	2		1	6		4	4	4			
Switch Phase												
Minimum Initial (s)	10.0	10.0		3.0	10.0		8.0	8.0	8.0			
Minimum Split (s)	18.0	18.0		8.3	18.0		16.0	16.0	16.0			
Total Split (s)	94.0	94.0		19.0	113.0		37.0	37.0	37.0			
Total Split (%)	62.7%	62.7%		12.7%	75.3%		24.7%	24.7%	24.7%			
Maximum Green (s)	86.0	86.0		13.7	105.0		29.0	29.0	29.0			
Yellow Time (s)	5.0	5.0		3.2	5.0		4.0	4.0	4.0			
All-Red Time (s)	3.0	3.0		2.1	3.0		4.0	4.0	4.0			
Lost Time Adjust (s)	0.0	0.0		0.0	0.0			0.0	0.0			
Total Lost Time (s)	8.0	8.0		5.3	8.0			8.0	8.0			
Lead/Lag	Lag	Lag		Lead								
Lead-Lag Optimize?	Yes	Yes		Yes								
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0	3.0			
Recall Mode	C-Max	C-Max		None	C-Max		None	None	None			
Walk Time (s)	7.0	7.0					7.0	7.0	7.0			
Flash Dont Walk (s)	25.0	25.0					22.0	22.0	22.0			
Pedestrian Calls (#/hr)	0	0		404.0	100 5		0	0	0			
Act Effct Green (s)		126.1		131.6	130.5			8.3	8.3			
Actuated g/C Ratio		0.84		0.88	0.87			0.06	0.06			
v/c Ratio		0.58		0.07	0.47			0.20	0.17			
Control Delay		6.4		0.8	0.9			72.5	2.4			
Queue Delay		0.2		0.0	0.5			0.0	0.5			
Total Delay		6.6		0.8	1.5			72.5	2.9			
LOS Annach Dalau		A		A	A			E	A			
Approach Delay		6.6			1.5			31.6				
Approach LOS		A			A			С				
Intersection Summary												
Area Type:	Other											
Cycle Length: 150	-											
Actuated Cycle Length: 150		0 EDTI			<b>6</b> 1 ( )							
Offset: 98 (65%), Reference	ed to phase	e 2:EBTL a	and 6:WE	BTL, Start	t of Yellow							
Natural Cycle: 70												
Control Type: Actuated-Co	ordinated											
Maximum v/c Ratio: 0.58	1.0											
Intersection Signal Delay: 4		,			ntersection		0					
Intersection Capacity Utiliza	auon 05.1%	)		IC	CU Level o	Service						
Analysis Period (min) 15												

## Splits and Phases: 2: Clapham Avenue/East Site Driveway & Northern Boulevard (NYS Route 25A)



	٠	_	+		5	7
	-			-		
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	<u></u>	<u></u>	<b>↑</b> Ъ	<b></b>	ካካ	1
Traffic Volume (vph)	195	1350	1014	305	395	202
Future Volume (vph)	195	1350	1014	305	395	202
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	110			0	435	115
Storage Lanes	1			0	1	1
Taper Length (ft)	25				25	
Lane Util. Factor	1.00	0.95	0.95	0.95	0.97	1.00
Frt			0.965			0.850
Flt Protected	0.950				0.950	
Satd. Flow (prot)	1752	3505	3382	0	3400	1568
Flt Permitted	0.950				0.950	
Satd. Flow (perm)	1752	3505	3382	0	3400	1568
Right Turn on Red				No		No
Satd. Flow (RTOR)						
Link Speed (mph)		35	35		30	
Link Distance (ft)		300	1113		956	
Travel Time (s)		5.8	21.7		21.7	
Peak Hour Factor	0.04			0.04		0.94
	0.94	0.94	0.94	0.94	0.94	
Heavy Vehicles (%)	3%	3%	3%	3%	3%	3%
Adj. Flow (vph)	207	1436	1079	324	420	215
Shared Lane Traffic (%)				_		
Lane Group Flow (vph)	207	1436	1403	0	420	215
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right
Median Width(ft)		12	12		24	
Link Offset(ft)		0	0		0	
Crosswalk Width(ft)		16	16		16	
Two way Left Turn Lane		Yes				
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15			9	15	9
Number of Detectors	1	2	2	J	1	1
Detector Template	Left	∠ Thru	Thru		Left	Right
Leading Detector (ft)	20	100	100		20	20
Trailing Detector (ft)	0	0	0		0	0
Detector 1 Position(ft)	0	0	0		0	0
Detector 1 Size(ft)	20	6	6		20	20
Detector 1 Type	CI+Ex	Cl+Ex	CI+Ex		Cl+Ex	Cl+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0		0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0		0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0		0.0	0.0
Detector 2 Position(ft)		94	94			
Detector 2 Size(ft)		6	6			
Detector 2 Type		Cl+Ex	CI+Ex			
Detector 2 Channel						
Detector 2 Extend (s)		0.0	0.0			
Turn Type	Prot	NA	NA		Prot	Over
Protected Phases		2	6		7	
	5	2	0		1	5

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Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Permitted Phases	_					
Detector Phase	5	2	6		7	5
Switch Phase						
Minimum Initial (s)	8.0	10.0	10.0		8.0	8.0
Minimum Split (s)	14.5	17.0	36.0		14.5	14.5
Total Split (s)	29.0	82.0	82.0		29.0	29.0
Total Split (%)	20.7%	58.6%	58.6%		20.7%	20.7%
Maximum Green (s)	22.5	75.0	75.0		22.5	22.5
Yellow Time (s)	4.0	5.0	5.0		4.0	4.0
All-Red Time (s)	2.5	2.0	2.0		2.5	2.5
Lost Time Adjust (s)	0.0	0.0	0.0		0.0	0.0
Total Lost Time (s)	6.5	7.0	7.0		6.5	6.5
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0	3.0	3.0		3.0	3.0
Recall Mode	None	C-Max	C-Max		None	None
Walk Time (s)	7.0		7.0		7.0	7.0
Flash Dont Walk (s)	21.0		22.0		22.0	21.0
Pedestrian Calls (#/hr)	0		0		0	0
Act Effct Green (s)	21.6	77.4	77.4		21.0	21.6
Actuated g/C Ratio	0.15	0.55	0.55		0.15	0.15
v/c Ratio	0.77	0.74	0.75		0.83	0.89
Control Delay	78.3	22.9	27.7		71.8	93.0
Queue Delay	13.1	0.6	0.0		0.0	0.0
Total Delay	91.5	23.5	27.7		71.8	93.0
LOS	F	С	С		E	F
Approach Delay		32.1	27.7		79.0	
Approach LOS		С	С		E	
Intersection Summary						
Area Type:	Other					
Cycle Length: 140						
Actuated Cycle Length: 14	0					
Offset: 27 (19%), Reference	ced to phase	e 2:EBT a	nd 6:WBT	, Start of	Yellow	
Natural Cycle: 90						
Control Type: Actuated-Co	oordinated					
Maximum v/c Ratio: 0.89						
Intersection Signal Delay:					ntersectio	
Intersection Capacity Utiliz	zation 76.5%	)		IC	CU Level	of Service
Analysis Period (min) 15						
Splits and Phases: 1: N	orthern Boul	evard (N)	YS Route	25A) & P	landome	Road



2: Clapham Avenue	•	Site Dri	iveway	/ & No	rthern	Boule	/ard (N	IYS Ro	oute 25	δA)	Ju	ne 2022
	٦	-	$\mathbf{F}$	4	+	*	•	1	۲	1	Ļ	~
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	<u>۲</u>	A1⊅		٦	<b>≜</b> î≽			र्च	1			
Traffic Volume (vph)	0	1533	9	9	1207	0	11	Ō	11	0	0	0
Future Volume (vph)	0	1533	9	9	1207	0	11	0	11	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	90		0	130		0	170		0	0		0
Storage Lanes	1		0	1		0	1		1	0		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.999							0.850			
Flt Protected				0.950				0.950				
Satd. Flow (prot)	1845	3501	0	1752	3505	0	0	1752	1568	0	0	0
Flt Permitted				0.119				0.950				
Satd. Flow (perm)	1845	3501	0	220	3505	0	0	1752	1568	0	0	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		1							80			
Link Speed (mph)		35			35			30			30	
Link Distance (ft)		183			300			662			430	
Travel Time (s)		3.6			5.8			15.0			9.8	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%
Adj. Flow (vph)	0	1648	10	10	1298	0	12	0	12	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	1658	0	10	1298	0	0	12	12	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane		Yes			Yes							
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2		1	2		1	2	1			
Detector Template	Left	Thru		Left	Thru		Left	Thru	Right			
Leading Detector (ft)	20	100		20	100		20	100	20			
Trailing Detector (ft)	0	0		0	0		0	0	0			
Detector 1 Position(ft)	0	0		0	0		0	0	0			
Detector 1 Size(ft)	20	6		20	6		20	6	20			
Detector 1 Type	Cl+Ex	CI+Ex		Cl+Ex	CI+Ex		Cl+Ex	CI+Ex	Cl+Ex			
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0			
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0			
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0			
Detector 2 Position(ft)		94			94			94				
Detector 2 Size(ft)		6			6			6				
Detector 2 Type		Cl+Ex			Cl+Ex			CI+Ex				
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0				
Turn Type	Perm	NA		pm+pt	NA		Perm	NA	Perm			
Protected Phases		2		1	6			4				

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# Lanes, Volumes, Timings

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Permitted Phases	2			6			4		4				
Detector Phase	2	2		1	6		4	4	4				
Switch Phase													
Minimum Initial (s)	10.0	10.0		3.0	10.0		8.0	8.0	8.0				
Minimum Split (s)	18.0	18.0		8.3	18.0		16.0	16.0	16.0				
Total Split (s)	77.0	77.0		19.0	96.0		44.0	44.0	44.0				
Total Split (%)	55.0%	55.0%		13.6%	68.6%		31.4%	31.4%	31.4%				
Maximum Green (s)	69.0	69.0		13.7	88.0		36.0	36.0	36.0				
Yellow Time (s)	5.0	5.0		3.2	5.0		4.0	4.0	4.0				
All-Red Time (s)	3.0	3.0		2.1	3.0		4.0	4.0	4.0				
Lost Time Adjust (s)	0.0	0.0		0.0	0.0			0.0	0.0				
Total Lost Time (s)	8.0	8.0		5.3	8.0			8.0	8.0				
Lead/Lag	Lag	Lag		Lead									
Lead-Lag Optimize?	Yes	Yes		Yes									
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0	3.0				
Recall Mode	C-Max	C-Max		None	C-Max		None	None	None				
Walk Time (s)	7.0	7.0					7.0	7.0	7.0				
Flash Dont Walk (s)	25.0	25.0					22.0	22.0	22.0				
Pedestrian Calls (#/hr)	0	0					0	0	0				
Act Effct Green (s)		123.3		125.1	125.6			8.0	8.0				
Actuated g/C Ratio		0.88		0.89	0.90			0.06	0.06				
v/c Ratio		0.54		0.04	0.41			0.12	0.07				
Control Delay		4.6		0.2	0.3			65.5	0.8				
Queue Delay		0.3		0.0	0.2			0.0	0.0				
Total Delay		4.9		0.2	0.5			65.5	0.8				
LOS		А		A	А			E	А				
Approach Delay		4.9			0.5			33.1					
Approach LOS		Α			Α			С					
Intersection Summary													
Area Type:	Other												
Cycle Length: 140													
Actuated Cycle Length: 14													
Offset: 36 (26%), Reference	ed to phase	e 2:EBTL a	and 6:WE	STL, Starl	of Yellow								
Natural Cycle: 65													
Control Type: Actuated-Co	ordinated												
Maximum v/c Ratio: 0.54													
Intersection Signal Delay:					ntersection								
Intersection Capacity Utiliz	ation 62.7%	)		10	CU Level o	of Service	B						
Analysis Period (min) 15													

2: Clapham Avenue/East Site Driveway & Northern Boulevard (NYS Route 25A) Splits and Phases:



	≯	-	+	•	· `	-
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
				WBR		
Lane Configurations		<b>*</b>	<b>†î</b>	404	<b>ካካ</b>	
Traffic Volume (vph)	153	840	991	404	494	167
Future Volume (vph)	153	840	991	404	494	167
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	110			0	435	115
Storage Lanes	1			0	1	1
Taper Length (ft)	25				25	
Lane Util. Factor	1.00	0.95	0.95	0.95	0.97	1.00
Frt			0.957			0.850
Flt Protected	0.950				0.950	
Satd. Flow (prot)	1752	3505	3354	0	3400	1568
Flt Permitted	0.950				0.950	
Satd. Flow (perm)	1752	3505	3354	0	3400	1568
Right Turn on Red			0001	No	0100	No
Satd. Flow (RTOR)						
Link Speed (mph)		35	35		30	
Link Opeed (mph)		300	1113		956	
( )		5.8	21.7			
Travel Time (s)	0.07			0.07	21.7	0.07
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Heavy Vehicles (%)	3%	3%	3%	3%	3%	3%
Adj. Flow (vph)	158	866	1022	416	509	172
Shared Lane Traffic (%)						
Lane Group Flow (vph)	158	866	1438	0	509	172
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right
Median Width(ft)		12	12		24	
Link Offset(ft)		0	0		0	
Crosswalk Width(ft)		16	16		16	
Two way Left Turn Lane		Yes				
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	1.00	1.00	1.00	9	15	9
Number of Detectors	1	2	2	5	13	1
Detector Template	Left	Z Thru	∠ Thru		Left	Right
Leading Detector (ft)	20	100	100		20	Right 20
Trailing Detector (ft)	0	0	0		0	0
Detector 1 Position(ft)	0	0	0		0	0
Detector 1 Size(ft)	20	6	6		20	20
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	CI+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0		0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0		0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0		0.0	0.0
Detector 2 Position(ft)		94	94			
Detector 2 Size(ft)		6	6			
Detector 2 Type		Cl+Ex	Cl+Ex			
Detector 2 Channel						
Detector 2 Extend (s)		0.0	0.0			
	Drot				Drot	Over
Turn Type	Prot	NA	NA		Prot	Over
Protected Phases	5	2	6		7	5

June 2	022
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Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Permitted Phases						
Detector Phase	5	2	6		7	5
Switch Phase						
Minimum Initial (s)	8.0	10.0	10.0		8.0	8.0
Minimum Split (s)	14.5	17.0	36.0		14.5	14.5
Total Split (s)	27.0	90.0	90.0		33.0	27.0
Total Split (%)	18.0%	60.0%	60.0%		22.0%	18.0%
Maximum Green (s)	20.5	83.0	83.0		26.5	20.5
Yellow Time (s)	4.0	5.0	5.0		4.0	4.0
All-Red Time (s)	2.5	2.0	2.0		2.5	2.5
Lost Time Adjust (s)	0.0	0.0	0.0		0.0	0.0
Total Lost Time (s)	6.5	7.0	7.0		6.5	6.5
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0	3.0	3.0		3.0	3.0
Recall Mode	None	C-Max	C-Max		None	None
Walk Time (s)	7.0		7.0		7.0	7.0
Flash Dont Walk (s)	21.0		22.0		22.0	21.0
Pedestrian Calls (#/hr)	0		0		0	0
Act Effct Green (s)	19.2	85.4	85.4		25.4	19.2
Actuated g/C Ratio	0.13	0.57	0.57		0.17	0.13
v/c Ratio	0.71	0.43	0.75		0.89	0.86
Control Delay	96.8	15.0	28.1		78.6	99.2
Queue Delay	7.1	0.4	0.0		0.0	0.0
Total Delay	103.9	15.4	28.1		78.6	99.2
LOS	F	В	С		E	F
Approach Delay		29.0	28.1		83.8	
Approach LOS		C	C		F	
		-	-			
Intersection Summary	• .:					
Area Type:	Other					
Cycle Length: 150	- ^					
Actuated Cycle Length: 1						
Offset: 14 (9%), Referenc	ed to phase	2:EBT an	d 6:WBT,	Start of \	rellow	
Natural Cycle: 90						
Control Type: Actuated-C	oordinated					
Maximum v/c Ratio: 0.89						
Intersection Signal Delay:						n LOS: D
Intersection Capacity Utili	zation 79.5%			IC	CU Level	of Service
Analysis Period (min) 15						
Splits and Phases: 1: N	lorthern Bou	levard (N`	YS Route	25A) & P	landome	Road
<b>V</b>		- \ -		1		
Ø5 27 s	→Ø2 (R) 90 s					

Ø5	→Ø2 (R)	<sup>1</sup> Ø7	
7 s	90 s	33 s	
	← Ø6 (R)		
	90 s		

Lanes, Volumes, Timings						
2: Clapham Avenue/East Site Driveway & Northern Bouleva	ard (N`	YS Rol	ite 25/	4)	June 20	022
A			-	,		_

	٦	-	$\mathbf{F}$	-	-	•	1	T.	1	1	Ŧ	-
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	<b>≜1</b> ≱		ሻ	<b>↑</b> Ъ			र्च	1			
Traffic Volume (vph)	4	936	10	17	1122	18	39	Ō	57	0	0	0
Future Volume (vph)	4	936	10	17	1122	18	39	0	57	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	90		0	130		0	170		0	0		0
Storage Lanes	1		0	1		0	1		1	0		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.998			0.998				0.850			
Flt Protected	0.950			0.950				0.950				
Satd. Flow (prot)	1752	3498	0	1752	3498	0	0	1752	1568	0	0	0
Flt Permitted	0.229	0.00	•	0.256		•	•	0.950		· ·	•	
Satd. Flow (perm)	422	3498	0	472	3498	0	0	1752	1568	0	0	0
Right Turn on Red		0100	Yes		0100	Yes	Ŭ		Yes	Ű	Ŭ	Yes
Satd. Flow (RTOR)		1	100		2	100			75			100
Link Speed (mph)		35			35			30	10		30	
Link Distance (ft)		183			300			662			430	
Travel Time (s)		3.6			5.8			15.0			9.8	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%
3	4	1006	11	18	1206	19	42	3 % 0	5 % 61	0	0	0
Adj. Flow (vph)	4	1006	11	10	1200	19	42	0	01	0	U	0
Shared Lane Traffic (%)	4	1017	0	18	1225	٥	0	42	61	0	0	0
Lane Group Flow (vph) Enter Blocked Intersection	4 No		No	No	No	0 No	0 No	42 No	No	No	No	No
		No										
Lane Alignment	Left	Left	Right	Left	Left 12	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12						0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane	4.00	Yes	4.00	4.00	Yes	4.00	4.00	4.00	4 00	4.00	4.00	4.00
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	0	9	15	0	9	15	0	9	15		9
Number of Detectors	1	2		1	2		1	2	1			
Detector Template	Left	Thru		Left	Thru		Left	Thru	Right			_
Leading Detector (ft)	20	100		20	100		20	100	20			
Trailing Detector (ft)	0	0		0	0		0	0	0			_
Detector 1 Position(ft)	0	0		0	0		0	0	0			
Detector 1 Size(ft)	20	6		20	6		20	6	20			
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	CI+Ex			
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0			
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0			
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0			
Detector 2 Position(ft)		94			94			94				
Detector 2 Size(ft)		6			6			6				
Detector 2 Type		Cl+Ex			Cl+Ex			CI+Ex				
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0				
Turn Type	Perm	NA		pm+pt	NA		Perm	NA	Perm			
Protected Phases		2			6			4				

2: Clapham Avenu	-	Site Dr	iveway	/ & No	rthern I	Boule	vard (N	IYS Ro	oute 25	A)	Jur	ne 2022
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Permitted Phases	2			6			4		4			
Detector Phase	2	2		1	6		4	4	4			
Switch Phase												
Minimum Initial (s)	10.0	10.0		3.0	10.0		8.0	8.0	8.0			
Minimum Split (s)	18.0	18.0		8.3	18.0		16.0	16.0	16.0			
Total Split (s)	95.0	95.0		18.0	113.0		37.0	37.0	37.0			
Total Split (%)	63.3%	63.3%		12.0%	75.3%		24.7%	24.7%	24.7%			
Maximum Green (s)	87.0	87.0		12.7	105.0		29.0	29.0	29.0			
Yellow Time (s)	5.0	5.0		3.2	5.0		4.0	4.0	4.0			
All-Red Time (s)	3.0	3.0		2.1	3.0		4.0	4.0	4.0			
Lost Time Adjust (s)	0.0	0.0		0.0	0.0			0.0	0.0			
Total Lost Time (s)	8.0	8.0		5.3	8.0			8.0	8.0			
Lead/Lag	Lag	Lag		Lead								
Lead-Lag Optimize?	Yes	Yes		Yes								
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0	3.0			
Recall Mode	C-Max	C-Max		None	C-Max		None	None	None			
Walk Time (s)	7.0	7.0					7.0	7.0	7.0			
Flash Dont Walk (s)	25.0	25.0					22.0	22.0	22.0			
Pedestrian Calls (#/hr)	0	0					0	0	0			
Act Effct Green (s)	122.5	122.5		130.3	129.2			9.6	9.6			
Actuated g/C Ratio	0.82	0.82		0.87	0.86			0.06	0.06			
v/c Ratio	0.01	0.36		0.04	0.41			0.38	0.36			
Control Delay	5.2	5.5		0.4	0.6			76.4	14.5			
Queue Delay	0.0	0.0		0.0	0.4			0.0	0.1			
Total Delay	5.2	5.5		0.4	1.0			76.4	14.6			
LOS	А	А		А	А			E	В			
Approach Delay		5.5			1.0			39.8				
Approach LOS		А			А			D				
Intersection Summary												
Area Type:	Other											
Cycle Length: 150												
Actuated Cycle Length: 15	50											
Offset: 39 (26%), Reference	ced to phase	e 2:EBTL a	and 6:WE	BTL, Star	of Yellow							
Natural Cycle: 55	a mallar a famil											
Control Type: Actuated-Co	oordinated											
Maximum v/c Ratio: 0.41	1.0					1.00						
Intersection Signal Delay:		,			ntersectior							
Intersection Capacity Utiliz	zation 51.6%	)		[(	CU Level o	of Service	A					
Analysis Period (min) 15												

## Splits and Phases: 2: Clapham Avenue/East Site Driveway & Northern Boulevard (NYS Route 25A)



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Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
				VVDR		
Lane Configurations	170	<b>††</b>	<b>†</b>	070	<u>ካ</u> ካ	170
Traffic Volume (vph)	170	1242	1005	270	504	179
Future Volume (vph)	170	1242	1005	270	504	179
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	110			0	435	115
Storage Lanes	1			0	1	1
Taper Length (ft)	25				25	
Lane Util. Factor	1.00	0.95	0.95	0.95	0.97	1.00
Frt			0.968			0.850
Flt Protected	0.950				0.950	
Satd. Flow (prot)	1752	3505	3393	0	3400	1568
Flt Permitted	0.950				0.950	
Satd. Flow (perm)	1752	3505	3393	0	3400	1568
Right Turn on Red				No	0100	No
Satd. Flow (RTOR)						
Link Speed (mph)		35	35		30	
Link Distance (ft)		300	1113		956	
( )		5.8	21.7			
Travel Time (s)	0.00			0.00	21.7	0.00
Peak Hour Factor	0.89	0.89	0.89	0.89	0.89	0.89
Heavy Vehicles (%)	3%	3%	3%	3%	3%	3%
Adj. Flow (vph)	191	1396	1129	303	566	201
Shared Lane Traffic (%)						
Lane Group Flow (vph)	191	1396	1432	0	566	201
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right
Median Width(ft)		12	12		24	
Link Offset(ft)		0	0		0	
Crosswalk Width(ft)		16	16		16	
Two way Left Turn Lane		Yes				
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	1.00		9	15	9
Number of Detectors	13	2	2	5	13	1
Detector Template	Left	Thru	Thru		Left	Right
Leading Detector (ft)	20	100	100		20	Right 20
Trailing Detector (ft)	0	0	0		0	0
Detector 1 Position(ft)	0	0	0		0	0
Detector 1 Size(ft)	20	6	6		20	20
Detector 1 Type	CI+Ex	Cl+Ex	Cl+Ex		Cl+Ex	CI+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0		0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0		0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0		0.0	0.0
Detector 2 Position(ft)		94	94			
Detector 2 Size(ft)		6	6			
Detector 2 Type		Cl+Ex	Cl+Ex			
Detector 2 Channel						
Detector 2 Extend (s)		0.0	0.0			
	Drot		NA		Drot	Over
Turn Type	Prot	NA			Prot	Over
Protected Phases	5	2	6		7	5

Build Midday AK

June 2	2022
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Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Permitted Phases						
Detector Phase	5	2	6		7	5
Switch Phase						
Minimum Initial (s)	8.0	10.0	10.0		8.0	8.0
Minimum Split (s)	14.5	17.0	36.0		14.5	14.5
Total Split (s)	29.0	82.0	82.0		29.0	29.0
Total Split (%)	20.7%	58.6%	58.6%		20.7%	20.7%
Maximum Green (s)	22.5	75.0	75.0		22.5	22.5
Yellow Time (s)	4.0	5.0	5.0		4.0	4.0
All-Red Time (s)	2.5	2.0	2.0		2.5	2.5
Lost Time Adjust (s)	0.0	0.0	0.0		0.0	0.0
Total Lost Time (s)	6.5	7.0	7.0		6.5	6.5
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0	3.0	3.0		3.0	3.0
Recall Mode	None	C-Max	C-Max		None	None
Walk Time (s)	7.0		7.0		7.0	7.0
Flash Dont Walk (s)	21.0		22.0		22.0	21.0
Pedestrian Calls (#/hr)	0		0		0	0
Act Effct Green (s)	20.8	76.7	76.7		22.5	20.8
Actuated g/C Ratio	0.15	0.55	0.55		0.16	0.15
v/c Ratio	0.73	0.73	0.77		1.04	0.86
Control Delay	85.6	20.5	28.7		104.6	90.0
Queue Delay	8.2	0.5	0.0		0.0	0.0
Total Delay	93.8	21.0	28.7		104.6	90.0
LOS	F	С	С		F	F
Approach Delay		29.8	28.7		100.8	
Approach LOS		С	С		F	
Intersection Summary						
Area Type:	Other					
Cycle Length: 140						
Actuated Cycle Length: 14						
Offset: 27 (19%), Referen	ced to phase	e 2:EBT a	nd 6:WBT	, Start of	Yellow	
Natural Cycle: 90						
Control Type: Actuated-Co	oordinated					
Maximum v/c Ratio: 1.04						
Intersection Signal Delay:					itersectio	
Intersection Capacity Utiliz	zation 76.9%	)		IC	CU Level	of Service
Analysis Period (min) 15						
Splits and Phases: 1: N	orthern Boul	evard (N	YS Route	25A) & P	landome	Road
						1,000
<b>\$</b> Ø5	-+02	(R)				



Lanes, Volumes, Timings	
2: Clapham Avenue/East Site Driveway & Northern Boulevard (NYS Route 25A)	June 2022

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	٦	<b>∱</b> î,		۲	A⊅			र्भ	1			
Traffic Volume (vph)	4	1353	27	26	1146	12	32	Ō	60	0	0	0
Future Volume (vph)	4	1353	27	26	1146	12	32	0	60	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	90		0	130		0	170		0	0		0
Storage Lanes	1		0	1		0	1		1	0		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.997			0.998				0.850			
Flt Protected	0.950			0.950				0.950				
Satd. Flow (prot)	1752	3494	0	1752	3498	0	0	1752	1568	0	0	0
Flt Permitted	0.218			0.136				0.950				
Satd. Flow (perm)	402	3494	0	251	3498	0	0	1752	1568	0	0	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		2			1				80			
Link Speed (mph)		35			35			30			30	
Link Distance (ft)		183			300			662			430	
Travel Time (s)		3.6			5.8			15.0			9.8	
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Heavy Vehicles (%)	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%
Adj. Flow (vph)	4	1487	30	29	1259	13	35	0	66	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	4	1517	0	29	1272	0	0	35	66	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12	Ŭ		12	Ŭ		0	Ŭ		0	Ŭ
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane		Yes			Yes							
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2		1	2		1	2	1			
Detector Template	Left	Thru		Left	Thru		Left	Thru	Right			
Leading Detector (ft)	20	100		20	100		20	100	20			
Trailing Detector (ft)	0	0		0	0		0	0	0			
Detector 1 Position(ft)	0	0		0	0		0	0	0			
Detector 1 Size(ft)	20	6		20	6		20	6	20			
Detector 1 Type	CI+Ex	CI+Ex		CI+Ex	CI+Ex		CI+Ex	CI+Ex	CI+Ex			
Detector 1 Channel		-		-								
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0			
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0			
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0			
Detector 2 Position(ft)		94			94			94				
Detector 2 Size(ft)		6			6			6				
Detector 2 Type		Cl+Ex			CI+Ex			Cl+Ex				
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0				
Turn Type	Perm	NA		pm+pt	NA		Perm	NA	Perm			
Protected Phases	1 0111	2		1 1	6		1 0111	4				
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-			am Avenue/East Site Driveway & Northern Boulevard (NYS Route 25A)									
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_ane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Permitted Phases	2			6			4		4			
Detector Phase	2	2		1	6		4	4	4			
Switch Phase												
Vinimum Initial (s)	10.0	10.0		3.0	10.0		8.0	8.0	8.0			
Vinimum Split (s)	18.0	18.0		8.3	18.0		16.0	16.0	16.0			
Total Split (s)	77.0	77.0		19.0	96.0		44.0	44.0	44.0			
Total Split (%)	55.0%	55.0%		13.6%	68.6%		31.4%	31.4%	31.4%			
Maximum Green (s)	69.0	69.0		13.7	88.0		36.0	36.0	36.0			
Yellow Time (s)	5.0	5.0		3.2	5.0		4.0	4.0	4.0			
All-Red Time (s)	3.0	3.0		2.1	3.0		4.0	4.0	4.0			
_ost Time Adjust (s)	0.0	0.0		0.0	0.0			0.0	0.0			
Total Lost Time (s)	8.0	8.0		5.3	8.0			8.0	8.0			
_ead/Lag	Lag	Lag		Lead								
_ead-Lag Optimize?	Yes	Yes		Yes								
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0	3.0			
Recall Mode	C-Max	C-Max		None	C-Max		None	None	None			
Walk Time (s)	7.0	7.0					7.0	7.0	7.0			
Flash Dont Walk (s)	25.0	25.0					22.0	22.0	22.0			
Pedestrian Calls (#/hr)	0	0					0	0	0			
Act Effct Green (s)	113.0	113.0		120.9	119.8			9.0	9.0			
Actuated g/C Ratio	0.81	0.81		0.86	0.86			0.06	0.06			
//c Ratio	0.01	0.54		0.10	0.42			0.31	0.38			
Control Delay	5.2	7.4		0.5	0.3			69.6	14.8			
Queue Delay	0.0	0.2		0.0	0.3			0.0	0.1			
Total Delay	5.2	7.6		0.5	0.6			69.6	14.8			
LOS	A	Α		A	А			E	В			
Approach Delay		7.6			0.6			33.8				
Approach LOS		Α			Α			С				
ntersection Summary												
Area Type: O	ther											
Cycle Length: 140												
Actuated Cycle Length: 140												
Offset: 36 (26%), Referenced	to phase	e 2:EBTL a	nd 6:WB	TL, Start	of Yellow							
Natural Cycle: 60												
Control Type: Actuated-Coord	dinated											
Vaximum v/c Ratio: 0.54												
ntersection Signal Delay: 5.4					ntersectior							
ntersection Capacity Utilization	on 58.3%	)		IC	CU Level o	of Service	B					
Analysis Period (min) 15												

2: Clapham Avenue/East Site Driveway & Northern Boulevard (NYS Route 25A) Splits and Phases:



Lane Group         EBL         EBT         WBT         WBR         SBL         SBR           Lane Configurations
Lane Configurations         Image: Configuration in the image: Configuratine in the image: Configuration in the image: Configuration in th
Lane Configurations         Image: Configuration in the image: Configuratine in the image: Configuration in the image: Configuration in th
Traffic Volume (vph)         188         1473         1233         329         504         169           Future Volume (vph)         188         1473         1233         329         504         169           Ideal Flow (vphpl)         1900         1900         1900         1900         1900         1900           Storage Length (ft)         110         0         435         115           Storage Lanes         1         0         1         1           Taper Length (ft)         25         25         25           Lane Util. Factor         1.00         0.95         0.95         0.97         1.00           Frt         0.968         0.850         0.850         0.850         0.850           Fit Protected         0.950         3393         0         3400         1568           Flt Permitted         0.950         0.950         0.950         0.950         0.950         0.950           Satd. Flow (perm)         1752         3505         3393         0         3400         1568           Right Turn on Red         No         No         No         Satd. Flow (RTOR)         No         No
Future Volume (vph)         188         1473         1233         329         504         169           Ideal Flow (vphpl)         1900         1900         1900         1900         1900         1900         1900           Storage Length (ft)         110         0         435         115           Storage Lanes         1         0         1         1           Taper Length (ft)         25         25         25           Lane Util. Factor         1.00         0.95         0.95         0.97         1.00           Frt         0.968         0.850         0.850         0.850         0.850         1568           Flt Protected         0.950         3393         0         3400         1568         1568           Flt Permitted         0.950         0.950         0.950         0.950         1568         1568           Flt Permitted         0.950         0.950         0.950         0.950         1568
Ideal Flow (vphpl)         1900         100         110         1700         1700         1700         1700         1700         1752         3505         3393         0         3400         1568         1568         1568         1568         1568         1568         1568         1568         1568         1568
Storage Length (ft)       110       0       435       115         Storage Lanes       1       0       1       1         Taper Length (ft)       25       25       25         Lane Util. Factor       1.00       0.95       0.95       0.97       1.00         Frt       0.968       0.850       0.850       0.850       0.850         Flt Protected       0.950       0.950       0.950       0.950       0.950         Satd. Flow (prot)       1752       3505       3393       0       3400       1568         Flt Permitted       0.950       0.950       0.950       0.950       0.950       0.950         Satd. Flow (perm)       1752       3505       3393       0       3400       1568         Right Turn on Red       No       No       No       No       Satd. Flow (RTOR)       No
Storage Lanes         1         0         1         1           Taper Length (ft)         25         25         25           Lane Util. Factor         1.00         0.95         0.95         0.97         1.00           Frt         0.968         0.850         0.850         0.850         0.850         0.850         0.850         0.950         0.950         0.950         0.950         5.095         0.950         5.095         0.950         5.095         0.950         5.095         5.095         0.950         5.095
Taper Length (ft)         25         25           Lane Util. Factor         1.00         0.95         0.95         0.97         1.00           Frt         0.968         0.850         0.850           Filt Protected         0.950         3393         0         3400         1568           Filt Permitted         0.950         0.950         0.950         5343. Flow (perm)         1752         3505         3393         0         3400         1568           Filt Permitted         0.950         0.950         0.950         0.950         5343. Flow (perm)         1752         3505         3393         0         3400         1568           Right Turn on Red         No         No         No         Satd. Flow (RTOR)         No         No
Lane Util. Factor         1.00         0.95         0.95         0.95         0.97         1.00           Frt         0.968         0.950         0.850         0.850         0.850         0.850         0.950         0.950         0.950         0.950         0.950         0.950         0.950         5340.         1568         Flt Permitted         0.950         0.950         0.950         5340.         1568         Flt Permitted         0.950         0.950         0.950         5340.         1568         Right Turn on Red         No         No         No         Satd. Flow (perm)         1752         3505         3393         0         3400         1568         No         No         Satd. Flow (perm)         1752         3505         3393         0         3400         1568         No         No         Satd. Flow (perm)         No         No         Satd. Flow (perm)         No         No         Satd. Flow (RTOR)         No         No         No         No         No         No         Satd. Flow (RTOR)         No         No <td< td=""></td<>
Frt         0.968         0.850           Flt Protected         0.950         0.950           Satd. Flow (prot)         1752         3505         3393         0         3400         1568           Flt Permitted         0.950         0.950         0.950         0.950           Satd. Flow (perm)         1752         3505         3393         0         3400         1568           Right Turn on Red         No         No         No         Satd. Flow (RTOR)         No
Flt Protected         0.950         0.950           Satd. Flow (prot)         1752         3505         3393         0         3400         1568           Flt Permitted         0.950         0.950         0.950         0.950           Satd. Flow (perm)         1752         3505         3393         0         3400         1568           Right Turn on Red         No         No         No         Satd. Flow (RTOR)         No
Satd. Flow (prot)         1752         3505         3393         0         3400         1568           Flt Permitted         0.950
Fit Permitted         0.950         0.950           Satd. Flow (perm)         1752         3505         3393         0         3400         1568           Right Turn on Red         No         No         No           Satd. Flow (RTOR)         Volume         No         No
Satd. Flow (perm)         1752         3505         3393         0         3400         1568           Right Turn on Red         No         No         No           Satd. Flow (RTOR)         Volume         No         No
Right Turn on Red No No Satd. Flow (RTOR)
Satd. Flow (RTOR)
Link Speed (mph) 35 35 30
Link Distance (ft) 300 1113 956
Travel Time (s) 5.8 21.7 21.7
Peak Hour Factor 0.98 0.98 0.98 0.98 0.98 0.98
Heavy Vehicles (%) 3% 3% 3% 3% 3% 3%
Adj. Flow (vph) 192 1503 1258 336 514 172
Shared Lane Traffic (%)
Lane Group Flow (vph) 192 1503 1594 0 514 172
Enter Blocked Intersection No No No No No No
Link Offset(ft) 0 0 0
Crosswalk Width(ft) 16 16 16
Two way Left Turn Lane Yes
Headway Factor 1.00 1.00 1.00 1.00 1.00 1.00
Turning Speed (mph)         15         9         15         9
Number of Detectors 1 2 2 1 1
Detector Template Left Thru Thru Left Right
Leading Detector (ft) 20 100 100 20 20
Trailing Detector (ft) 0 0 0 0 0
Detector 1 Position(ft) 0 0 0 0 0
Detector 1 Size(ft) 20 6 6 20 20
Detector 1 Type CI+Ex CI+Ex CI+Ex CI+Ex CI+Ex
Detector 1 Channel
Detector 1 Extend (s) 0.0 0.0 0.0 0.0 0.0
Detector 1 Queue (s) 0.0 0.0 0.0 0.0 0.0 0.0 0.0
<b>,</b> ( )
Detector 2 Position(ft) 94 94
Detector 2 Size(ft) 6 6
Detector 2 Type CI+Ex CI+Ex
Detector 2 Channel
Detector 2 Extend (s) 0.0 0.0
Turn Type Prot NA NA Prot Over
Protected Phases 5 2 6 7 5

June 2	2022
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Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Permitted Phases						
Detector Phase	5	2	6		7	5
Switch Phase						
Minimum Initial (s)	8.0	10.0	10.0		8.0	8.0
Minimum Split (s)	14.5	17.0	36.0		14.5	14.5
Total Split (s)	27.0	90.0	90.0		33.0	27.0
Total Split (%)	18.0%	60.0%	60.0%		22.0%	18.0%
Maximum Green (s)	20.5	83.0	83.0		26.5	20.5
Yellow Time (s)	4.0	5.0	5.0		4.0	4.0
All-Red Time (s)	2.5	2.0	2.0		2.5	2.5
Lost Time Adjust (s)	0.0	0.0	0.0		0.0	0.0
Total Lost Time (s)	6.5	7.0	7.0		6.5	6.5
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0	3.0	3.0		3.0	3.0
Recall Mode	None	C-Max	C-Max		None	None
Walk Time (s)	7.0		7.0		7.0	7.0
Flash Dont Walk (s)	21.0		22.0		22.0	21.0
Pedestrian Calls (#/hr)	0		0		0	0
Act Effct Green (s)	19.2	85.3	85.3		25.5	19.2
Actuated g/C Ratio	0.13	0.57	0.57		0.17	0.13
v/c Ratio	0.86	0.75	0.83		0.89	0.86
Control Delay	104.5	22.0	31.6		79.1	98.8
Queue Delay	27.1	0.7	0.0		0.0	0.0
Total Delay	131.7	22.8	31.6		79.1	98.8
LOS	F	С	С		E	F
Approach Delay		35.1	31.6		84.0	
Approach LOS		D	С		F	
Intersection Summary						
Area Type:	Other					
Cycle Length: 150	Ourier					
Actuated Cycle Length: 1	50					
Offset: 66 (44%), Referen		2.FRT a	nd 6·WBT	Start of	Yellow	
Natural Cycle: 90		2.CD1 a		, otari ol	10100	
Control Type: Actuated-C	oordinated					
Maximum v/c Ratio: 0.89						
Intersection Signal Delay:	42 1			Ir	ntersectio	
Intersection Capacity Utili						of Service
Analysis Period (min) 15	201011 00.0 /	J		K		
Splits and Phases: 1: N	Iorthern Bou	levard (N`	<u>YS Rout</u> e	25A) & P	landome	Road
2 as	- (p)					
Ø5	→Ø2 (R) 90 s					

Ø5	→Ø2 (R)		Ø7	
27 s	90 s		33 s	
	4- (75 (P)	_		
	06 (R)			
	90 s			

Lanes, Volumes, 1 2: Clapham Avenu	0	Site Dri	veway	<sup>,</sup> & Noi	thern	Boulev	ard (N	YS Ro	oute 25	A)	Jur	ne 2022
	٦	-	$\mathbf{F}$	•	-	•	1	1	1	1	ţ	~
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1	A1⊅		ľ	A1⊅			÷٩	1			
Traffic Volume (vph)	3	1635	14	16	1378	8	18	Ő	26	0	0	0
Future Volume (vph)	3	1635	14	16	1378	8	18	0	26	0	0	0

Traffic Volume (vpn)	3	1035	14	10	13/0	ō	10	U	20	U	0	U
Future Volume (vph)	3	1635	14	16	1378	8	18	0	26	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	90		0	130		0	170		0	0		0
Storage Lanes	1		0	1		0	1		1	0		0
Taper Length (ft)	25		v	25		Ū	25		•	25		Ŭ
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	0.95	1.00	1.00	1.00	1.00	1.00	1.00
	1.00		0.95	1.00		0.95	1.00	1.00		1.00	1.00	1.00
Frt	0.050	0.999		0.050	0.999			0.050	0.850			
Flt Protected	0.950			0.950				0.950				
Satd. Flow (prot)	1752	3501	0	1752	3501	0	0	1752	1568	0	0	0
Flt Permitted	0.182			0.108				0.950				
Satd. Flow (perm)	336	3501	0	199	3501	0	0	1752	1568	0	0	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		1			1				75			
Link Speed (mph)		35			35			30			30	
Link Distance (ft)		183			300			662			430	
Travel Time (s)		3.6			5.8			15.0			9.8	
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%
Heavy Vehicles (%)												
Adj. Flow (vph)	3	1703	15	17	1435	8	19	0	27	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	3	1718	0	17	1443	0	0	19	27	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane		Yes			Yes							
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	1.00	1.00	9	1.00	1.00	9	1.00	1.00	9	1.00	1.00	9
Number of Detectors	13	2	9	1	2	9	15	2	<u> </u>	15		9
Detector Template	Left	Thru		Left	Thru		Left	Thru	Right			_
Leading Detector (ft)	20	100		20	100		20	100	20			
Trailing Detector (ft)	0	0		0	0		0	0	0			
Detector 1 Position(ft)	0	0		0	0		0	0	0			
Detector 1 Size(ft)	20	6		20	6		20	6	20			
Detector 1 Type	CI+Ex	CI+Ex		Cl+Ex	CI+Ex		CI+Ex	CI+Ex	CI+Ex			
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0			
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0			
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0			
	0.0	94		0.0	0.0 94		0.0	94	0.0			
Detector 2 Position(ft)												
Detector 2 Size(ft)		6			6			6				
Detector 2 Type		Cl+Ex			Cl+Ex			CI+Ex				
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0				
Turn Type	Perm	NA		pm+pt	NA		Perm	NA	Perm			
Protected Phases		2		1	6			4				

2: Clapham Avenu			veway			Douie	aiu (i		Jule 20	(7	Jui	ne 2022
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Permitted Phases	2			6			4		4			
Detector Phase	2	2		1	6		4	4	4			
Switch Phase												
Minimum Initial (s)	10.0	10.0		3.0	10.0		8.0	8.0	8.0			
Minimum Split (s)	18.0	18.0		8.3	18.0		16.0	16.0	16.0			
Total Split (s)	94.0	94.0		19.0	113.0		37.0	37.0	37.0			
Total Split (%)	62.7%	62.7%		12.7%	75.3%		24.7%	24.7%	24.7%			
Maximum Green (s)	86.0	86.0		13.7	105.0		29.0	29.0	29.0			
Yellow Time (s)	5.0	5.0		3.2	5.0		4.0	4.0	4.0			
All-Red Time (s)	3.0	3.0		2.1	3.0		4.0	4.0	4.0			
Lost Time Adjust (s)	0.0	0.0		0.0	0.0			0.0	0.0			
Total Lost Time (s)	8.0	8.0		5.3	8.0			8.0	8.0			
Lead/Lag	Lag	Lag		Lead								
Lead-Lag Optimize?	Yes	Yes		Yes								
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0	3.0			
Recall Mode	C-Max	C-Max		None	C-Max		None	None	None			
Walk Time (s)	7.0	7.0					7.0	7.0	7.0			
Flash Dont Walk (s)	25.0	25.0					22.0	22.0	22.0			
Pedestrian Calls (#/hr)	0	0					0	0	0			
Act Effct Green (s)	126.1	126.1		131.6	130.5			8.3	8.3			
Actuated g/C Ratio	0.84	0.84		0.88	0.87			0.06	0.06			
v/c Ratio	0.01	0.58		0.07	0.47			0.20	0.17			
Control Delay	4.3	6.5		0.8	0.9			72.5	2.4			
Queue Delay	0.0	0.2		0.0	0.6			0.0	0.5			
Total Delay	4.3	6.7		0.8	1.5			72.5	2.9			
LOS	А	А		A	А			E	А			
Approach Delay		6.7			1.5			31.6				
Approach LOS		А			Α			С				
Intersection Summary												
Area Type:	Other											
Cycle Length: 150												
Actuated Cycle Length: 15												
Offset: 98 (65%), Reference	ced to phase	e 2:EBTL a	and 6:WE	BTL, Start	of Yellow							
Natural Cycle: 70												
Control Type: Actuated-Co	ordinated											
Maximum v/c Ratio: 0.58												
Intersection Signal Delay:					ntersection							
Intersection Capacity Utiliz	ation 65.6%	0		10	CU Level o	ot Service	ЭC					
Analysis Period (min) 15												

## Splits and Phases: 2: Clapham Avenue/East Site Driveway & Northern Boulevard (NYS Route 25A)



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	-					000
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	<u></u>	<u></u>	<b>↑</b> Ъ	<b></b>	ካካ	1
Traffic Volume (vph)	200	1372	1035	305	395	207
Future Volume (vph)	200	1372	1035	305	395	207
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	110			0	435	115
Storage Lanes	1			0	1	1
Taper Length (ft)	25				25	
Lane Util. Factor	1.00	0.95	0.95	0.95	0.97	1.00
Frt			0.966			0.850
Flt Protected	0.950				0.950	
Satd. Flow (prot)	1752	3505	3386	0	3400	1568
Flt Permitted	0.950				0.950	
Satd. Flow (perm)	1752	3505	3386	0	3400	1568
Right Turn on Red				No		No
Satd. Flow (RTOR)						
Link Speed (mph)		35	35		30	
Link Distance (ft)		300	1113		956	
Travel Time (s)		5.8	21.7		21.7	
Peak Hour Factor	0.94	5.0 0.94	0.94	0.94	0.94	0.94
Heavy Vehicles (%)	3%	3%	3%	3%	3%	3%
Adj. Flow (vph)	213	1460	1101	324	420	220
Shared Lane Traffic (%)				_		
Lane Group Flow (vph)	213	1460	1425	0	420	220
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right
Median Width(ft)		12	12		24	
Link Offset(ft)		0	0		0	
Crosswalk Width(ft)		16	16		16	
Two way Left Turn Lane		Yes				
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	1.00		9	15	9
Number of Detectors	1	2	2	J	1	1
Detector Template	Left	Thru	∠ Thru		Left	Right
Leading Detector (ft)	20	100	100		20	20
Trailing Detector (ft)	0	0	0		0	0
Detector 1 Position(ft)	0	0	0		0	0
Detector 1 Size(ft)	20	6	6		20	20
Detector 1 Type	CI+Ex	Cl+Ex	Cl+Ex		Cl+Ex	CI+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0		0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0		0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0		0.0	0.0
Detector 2 Position(ft)		94	94			
Detector 2 Size(ft)		6	6			
Detector 2 Type		Cl+Ex	Cl+Ex			
Detector 2 Channel						
Detector 2 Extend (s)		0.0	0.0			
Turn Type	Prot	NA	NA		Prot	Over
Protected Phases		2	6		7	
	5	۷	0		1	5

Build Saturday AK

June 2	2022
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Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Permitted Phases						
Detector Phase	5	2	6		7	5
Switch Phase						-
Minimum Initial (s)	8.0	10.0	10.0		8.0	8.0
Minimum Split (s)	14.5	17.0	36.0		14.5	14.5
Total Split (s)	29.0	82.0	82.0		29.0	29.0
Total Split (%)	20.7%	58.6%	58.6%		20.7%	20.7%
Maximum Green (s)	22.5	75.0	75.0		22.5	22.5
Yellow Time (s)	4.0	5.0	5.0		4.0	4.0
All-Red Time (s)	2.5	2.0	2.0		2.5	2.5
Lost Time Adjust (s)	0.0	0.0	0.0		0.0	0.0
Total Lost Time (s)	6.5	7.0	7.0		6.5	6.5
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0	3.0	3.0		3.0	3.0
Recall Mode	None	C-Max	C-Max		None	None
Walk Time (s)	7.0		7.0		7.0	7.0
Flash Dont Walk (s)	21.0		22.0		22.0	21.0
Pedestrian Calls (#/hr)	0		0		0	0
Act Effct Green (s)	21.9	77.2	77.2		21.0	21.9
Actuated g/C Ratio	0.16	0.55	0.55		0.15	0.16
v/c Ratio	0.78	0.76	0.76		0.83	0.90
Control Delay	79.0	23.4	28.3		71.8	94.1
Queue Delay	14.5	0.8	0.0		0.0	0.0
Total Delay	93.5	24.2	28.3		71.8	94.1
LOS	F	С	С		E	F
Approach Delay		33.0	28.3		79.4	
Approach LOS		С	С		E	
Intersection Summary						
Area Type:	Other					
Cycle Length: 140						
Actuated Cycle Length: 14	0					
Offset: 27 (19%), Reference	ed to phase	e 2:EBT a	nd 6:WBT	, Start of	Yellow	
Natural Cycle: 90						
Control Type: Actuated-Co	ordinated					
Maximum v/c Ratio: 0.90						
Intersection Signal Delay:	39.2			Ir	ntersectio	n LOS: D
Intersection Capacity Utiliz	ation 77.4%	)		IC	CU Level	of Service
Analysis Period (min) 15						
						<b>_</b> .
	orthern Boul	evard (N	YS Route	25A) & P	landome	Road
<b>**</b> ar	-	(0)				

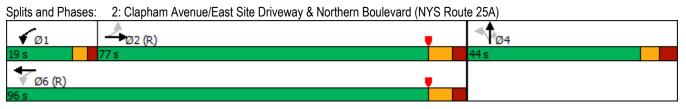


Lanes, Volumes, Timings	
2: Clapham Avenue/East Site Driveway & Northern Boulevard (NYS Route 25A)	June 2022

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	۲	A		۲	A			4	1			
Traffic Volume (vph)	5	1560	9	9	1215	19	11	Ō	11	0	0	0
Future Volume (vph)	5	1560	9	9	1215	19	11	0	11	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	90		0	130		0	170		0	0		0
Storage Lanes	1		0	1		0	1		1	0		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.95	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.999			0.998				0.850			
Flt Protected	0.950			0.950				0.950				
Satd. Flow (prot)	1752	3501	0	1752	3498	0	0	1752	1568	0	0	0
Flt Permitted	0.207			0.114				0.950				
Satd. Flow (perm)	382	3501	0	210	3498	0	0	1752	1568	0	0	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		1			2				80			
Link Speed (mph)		35			35			30			30	
Link Distance (ft)		183			300			662			430	
Travel Time (s)		3.6			5.8			15.0			9.8	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%	3%
Adj. Flow (vph)	5	1677	10	10	1306	20	12	0	12	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	5	1687	0	10	1326	0	0	12	12	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12	Ŭ		12	Ŭ		0	Ŭ		0	Ŭ
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane		Yes			Yes							
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2		1	2		1	2	1			
Detector Template	Left	Thru		Left	Thru		Left	Thru	Right			
Leading Detector (ft)	20	100		20	100		20	100	20			
Trailing Detector (ft)	0	0		0	0		0	0	0			
Detector 1 Position(ft)	0	0		0	0		0	0	0			
Detector 1 Size(ft)	20	6		20	6		20	6	20			
Detector 1 Type	CI+Ex	CI+Ex		Cl+Ex	Cl+Ex		CI+Ex	CI+Ex	CI+Ex			
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0			
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0			
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0			
Detector 2 Position(ft)		94			94			94				
Detector 2 Size(ft)		6			6			6				
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex				
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0				
Turn Type	Perm	NA		pm+pt	NA		Perm	NA	Perm			
Protected Phases		2		1	6			4				
		-			•			•				

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBI
Permitted Phases	2			6			4		4			
Detector Phase	2	2		1	6		4	4	4			
Switch Phase												
Minimum Initial (s)	10.0	10.0		3.0	10.0		8.0	8.0	8.0			
Minimum Split (s)	18.0	18.0		8.3	18.0		16.0	16.0	16.0			
Total Split (s)	77.0	77.0		19.0	96.0		44.0	44.0	44.0			
Total Split (%)	55.0%	55.0%		13.6%	68.6%		31.4%	31.4%	31.4%			
Maximum Green (s)	69.0	69.0		13.7	88.0		36.0	36.0	36.0			
Yellow Time (s)	5.0	5.0		3.2	5.0		4.0	4.0	4.0			
All-Red Time (s)	3.0	3.0		2.1	3.0		4.0	4.0	4.0			
Lost Time Adjust (s)	0.0	0.0		0.0	0.0			0.0	0.0			
Total Lost Time (s)	8.0	8.0		5.3	8.0			8.0	8.0			
Lead/Lag	Lag	Lag		Lead								
Lead-Lag Optimize?	Yes	Yes		Yes								
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0	3.0			
Recall Mode	C-Max	C-Max		None	C-Max		None	None	None			
Walk Time (s)	7.0	7.0					7.0	7.0	7.0			
Flash Dont Walk (s)	25.0	25.0					22.0	22.0	22.0			
Pedestrian Calls (#/hr)	0	0					0	0	0			
Act Effct Green (s)	123.3	123.3		125.1	125.6			8.0	8.0			
Actuated g/C Ratio	0.88	0.88		0.89	0.90			0.06	0.06			
v/c Ratio	0.01	0.55		0.04	0.42			0.12	0.07			
Control Delay	3.6	4.7		0.2	0.3			65.5	0.8			
Queue Delay	0.0	0.3		0.0	0.2			0.0	0.0			
Total Delay	3.6	5.0		0.2	0.5			65.5	0.8			
LOS	А	А		А	А			Е	А			
Approach Delay		5.0			0.5			33.1				
Approach LOS		А			А			С				
Intersection Summary												
Area Type:	Other											
Cycle Length: 140												
Actuated Cycle Length: 14												
Offset: 36 (26%), Reference	ced to phase	e 2:EBTL a	and 6:WB	STL, Start	of Yellow							
Natural Cycle: 65												
Control Type: Actuated-Co	ordinated											
Maximum v/c Ratio: 0.55												
Intersection Signal Delay:					ntersectior							
Intersection Capacity Utiliz	ation 63.4%	'n		10	CU Level o	of Service	B					



## UNSIGNALIZED INTERSECTIONS

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### Intersection

Int Delay, s/veh

,						
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	ľ	- <b>†</b> †	Aî∌		Y	
Traffic Vol, veh/h	42	764	1082	13	9	94
Future Vol, veh/h	42	764	1082	13	9	94
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	100	-	-	-	0	-
Veh in Median Storage,	, # -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	93	93	93	93	93	93
Heavy Vehicles, %	3	3	3	3	3	3
Mvmt Flow	45	822	1163	14	10	101

Major/Minor	Major1	Ν	lajor2		Vinor2	
Conflicting Flow All	1177	0	-		1671	589
Stage 1	-	-	-	-	1170	-
Stage 2	-	-	-	-	501	-
Critical Hdwy	4.16	-	-	-	6.86	6.96
Critical Hdwy Stg 1	-	-	-	-	5.86	-
Critical Hdwy Stg 2	-	-	-	-	5.86	-
Follow-up Hdwy	2.23	-	-	-	3.53	3.33
Pot Cap-1 Maneuver	584	-	-	-	86	449
Stage 1	-	-	-	-	255	-
Stage 2	-	-	-	-	571	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver		-	-	-	79	449
Mov Cap-2 Maneuver	r -	-	-	-	180	-
Stage 1	-	-	-	-	235	-
Stage 2	-	-	-	-	571	-
Approach	EB		WB		SB	
HCM Control Delay, s	s 0.6		0		17.5	
HCM LOS					С	
Minor Lane/Major Mv	mt	EBL	EBT	WBT	WBR S	SBLn1
Capacity (veh/h)		584	-	-	-	397
HCM Lane V/C Ratio		0.077	-	-	-	0.279
HCM Control Delay (s	s)	11.7	-	-	-	17.5
HCM Lane LOS		В	-	-	-	С
HCM 95th %tile Q(ve	h)	0.3	-	-	-	1.1

#### Intersection Int Delay, s/veh 0.9 EBL EBT WBT WBR SBL SBR Movement Lane Configurations ٦ **↑**↑ ۴Þ ¥ 1024 Traffic Vol, veh/h 45 1189 17 11 83 Future Vol, veh/h 45 1189 1024 17 11 83 Conflicting Peds, #/hr 0 0 0 0 0 0 Sign Control Stop Free Free Free Free Stop RT Channelized -None -None -None Storage Length 100 0 -\_ --Veh in Median Storage, # -0 0 -0 -Grade, % 0 0 0 ---Peak Hour Factor 92 92 92 92 92 92 Heavy Vehicles, % 3 3 3 3 3 3 Mvmt Flow 49 1292 1113 18 12 90

Major/Minor	Major1	Ν	lajor2	1	Minor2	
Conflicting Flow All	1131	0	-	0	1866	566
Stage 1	-	-	-	-	1122	-
Stage 2	-	-	-	-	744	-
Critical Hdwy	4.16	-	-	-	6.86	6.96
Critical Hdwy Stg 1	-	-	-	-	5.86	-
Critical Hdwy Stg 2	-	-	-	-	5.86	-
Follow-up Hdwy	2.23	-	-	-	3.53	3.33
Pot Cap-1 Maneuver	608	-	-	-	63	465
Stage 1	-	-	-	-	271	-
Stage 2	-	-	-	-	428	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuve		-	-	-	58	465
Mov Cap-2 Maneuve	r –	-	-	-	168	-
Stage 1	-	-	-	-	249	-
Stage 2	-	-	-	-	428	-
Approach	EB		WB		SB	
HCM Control Delay, s	s 0.4		0		17.7	
HCM LOS					С	
Minor Lane/Major Mv	mt	EBL	EBT	WBT	WBR S	SBLn1
Capacity (veh/h)		608	-	-	-	385
HCM Lane V/C Ratio		0.08	-	-	-	0.265
HCM Control Delay (s	s)	11.4	-	-	-	17.7
HCM Lane LOS						
	/	В	-	-	-	С

Int Delay, s/veh	0.8					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	٦	- 11	_ <b>≜</b> î≽		Y	
Traffic Vol, veh/h	60	1502	1165	24	4	89
Future Vol, veh/h	60	1502	1165	24	4	89
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	100	-	-	-	0	-
Veh in Median Storage	, # -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	3	3	3	3	3	3
Mvmt Flow	63	1581	1226	25	4	94

Major/Minor	Major1	Ν	/lajor2		Minor2	
Conflicting Flow All	1251	0	-	0	2156	626
Stage 1	-	-	-	-	1239	-
Stage 2	-	-	-	-	917	-
Critical Hdwy	4.16	-	-	-	6.86	6.96
Critical Hdwy Stg 1	-	-	-	-	5.86	-
Critical Hdwy Stg 2	-	-	-	-	5.86	-
Follow-up Hdwy	2.23	-	-	-	3.53	3.33
Pot Cap-1 Maneuver	547	-	-	-	40	425
Stage 1	-	-	-	-	234	-
Stage 2	-	-	-	-	348	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver		-	-	-	35	425
Mov Cap-2 Maneuver	-	-	-	-	133	-
Stage 1	-	-	-	-	207	-
Stage 2	-	-	-	-	348	-
Approach	EB		WB		SB	
HCM Control Delay, s	0.5		0		17.4	
HCM LOS					С	
Minor Lane/Major Mvn	nt	EBL	EBT	WBT	WBR	2DIn1
	IIL		EDI	VVDI	VVDR	
Capacity (veh/h) HCM Lane V/C Ratio		547	-	-	-	388
	۱ ۱	0.115 12.4	-	-	-	0.252
HCM Control Delay (s HCM Lane LOS	)	12.4 B	-	-	-	17.4 C
HCM 95th %tile Q(veh	.)	0.4	-	-	-	1
	1)	0.4	-	-	-	

Int Delay, s/veh	0.7					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	- ኘ	- 11	_ <b>≜</b> î≽		۰¥	
Traffic Vol, veh/h	63	1378	1082	12	5	67
Future Vol, veh/h	63	1378	1082	12	5	67
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	100	-	-	-	0	-
Veh in Median Storage	, # -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	94	94	94	94	94	94
Heavy Vehicles, %	3	3	3	3	3	3
Mvmt Flow	67	1466	1151	13	5	71

Major/Minor	Major1	N	/lajor2		Minor2	
Conflicting Flow All	1164	0	-		2025	582
Stage 1	-	-	-	-	1158	-
Stage 2	-	-	-	-	867	-
Critical Hdwy	4.16	-	-	-	6.86	6.96
Critical Hdwy Stg 1	-	-	-	-	5.86	-
Critical Hdwy Stg 2	-	-	-	-	5.86	-
Follow-up Hdwy	2.23	-	-	-	3.53	3.33
Pot Cap-1 Maneuver	590	-	-	-	50	454
Stage 1	-	-	-	-	259	-
Stage 2	-	-	-	-	369	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver		-	-	-	44	454
Mov Cap-2 Maneuver	r -	-	-	-	148	-
Stage 1	-	-	-	-	229	-
Stage 2	-	-	-	-	369	-
Approach	EB		WB		SB	
HCM Control Delay, s	s 0.5		0		16.2	
HCM LOS					С	
Minor Lane/Major Mv	mt	EBL	EBT	WBT	WBR S	SBLn1
Capacity (veh/h)		590			-	397
HCM Lane V/C Ratio		0.114	-	-		0.193
HCM Control Delay (s		11.9	-	-	_	16.2
HCM Lane LOS	-,	B	-	-	-	C
HCM 95th %tile Q(ve	h)	0.4	-	-	-	0.7
	/					

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# Intersection

,						
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	ľ	<b>^</b>	<b>∱</b> î,		Y	
Traffic Vol, veh/h	43	928	1143	13	9	96
Future Vol, veh/h	43	928	1143	13	9	96
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	100	-	-	-	0	-
Veh in Median Storage,	, # -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	93	93	93	93	93	93
Heavy Vehicles, %	3	3	3	3	3	3
Mvmt Flow	46	998	1229	14	10	103

Major/Minor	Major1	Ν	1ajor2	I	Minor2	
Conflicting Flow All	1243	0	-	0	1827	622
Stage 1	-	-	-	-	1236	-
Stage 2	-	-	-	-	591	-
Critical Hdwy	4.16	-	-	-	6.86	6.96
Critical Hdwy Stg 1	-	-	-	-	5.86	-
Critical Hdwy Stg 2	-	-	-	-	5.86	-
Follow-up Hdwy	2.23	-	-	-	3.53	3.33
Pot Cap-1 Maneuver	550	-	-	-	67	427
Stage 1	-	-	-	-	235	-
Stage 2	-	-	-	-	513	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver		-	-	-	61	427
Mov Cap-2 Maneuver		-	-	-	161	-
Stage 1	-	-	-	-	215	-
Stage 2	-	-	-	-	513	-
Approach	EB		WB		SB	
HCM Control Delay, s	0.5		0		18.7	
HCM LOS					С	
Minor Lane/Major Mvr	mt	EBL	EBT	WBT	WBR S	SBLn1
Capacity (veh/h)		550				374
HCM Lane V/C Ratio		0.084	-	-	-	0.302
HCM Control Delay (s	3)	12.1	-	-	-	18.7
HCM Lane LOS	,	B	-	-	-	C
HCM 95th %tile Q(vel	h)	0.3	-	-	-	1.2
	/					

Int Delay, s/veh	0.9						
Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations	۲.	- 11	_ <b>≜</b> î≽		Y		
Traffic Vol, veh/h	46	1348	1155	17	11	85	
Future Vol, veh/h	46	1348	1155	17	11	85	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Stop	Stop	
RT Channelized	-	None	-	None	-	None	
Storage Length	100	-	-	-	0	-	
Veh in Median Storage,	# -	0	0	-	0	-	
Grade, %	-	0	0	-	0	-	
Peak Hour Factor	92	92	92	92	92	92	
Heavy Vehicles, %	3	3	3	3	3	3	
Mvmt Flow	50	1465	1255	18	12	92	

Major/Minor	Major1	Ν	lajor2		Minor2	
Conflicting Flow All	1273	0	-	0	2097	637
Stage 1	-	-	-	-	1264	-
Stage 2	-	-	-	-	833	-
Critical Hdwy	4.16	-	-	-	6.86	6.96
Critical Hdwy Stg 1	-	-	-	-	5.86	-
Critical Hdwy Stg 2	-	-	-	-	5.86	-
Follow-up Hdwy	2.23	-	-	-	3.53	3.33
Pot Cap-1 Maneuver	536	-	-	-	44	418
Stage 1	-	-	-	-	227	-
Stage 2	-	-	-	-	385	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver		-	-	-	40	418
Mov Cap-2 Maneuver	-	-	-	-	139	-
Stage 1	-	-	-	-	206	-
Stage 2	-	-	-	-	385	-
Approach	EB		WB		SB	
HCM Control Delay, s	0.4		0		20.2	
HCM LOS					С	
Minor Lane/Major Mvr	nt	EBL	EBT	WBT	WBR	SBI n1
Capacity (veh/h)		536			-	340
HCM Lane V/C Ratio		0.093		_		0.307
HCM Control Delay (s	)	12.4	_	_	_	20.2
HCM Lane LOS	/	B	-	-	_	20.2 C
HCM 95th %tile Q(veh	1)	0.3	-	-	-	1.3
	'/	0.0				1.0

Int Delay, s/veh	0.9					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	5	<b>^</b>	<b>∱</b> ₽		Y	
Traffic Vol, veh/h	61	1627	1368	24	4	91
Future Vol, veh/h	61	1627	1368	24	4	91
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	100	-	-	-	0	-
Veh in Median Storage	, # -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	3	3	3	3	3	3
Mvmt Flow	64	1713	1440	25	4	96

Major/Minor	Major1	Ν	lajor2	I	Minor2	
Conflicting Flow All	1465	0	-	0	2438	733
Stage 1	-	-	-	-	1453	-
Stage 2	-	-	-	-	985	-
Critical Hdwy	4.16	-	-	-	6.86	6.96
Critical Hdwy Stg 1	-	-	-	-	5.86	-
Critical Hdwy Stg 2	-	-	-	-	5.86	-
Follow-up Hdwy	2.23	-	-	-	3.53	3.33
Pot Cap-1 Maneuver	452	-	-	-	26	361
Stage 1	-	-	-	-	180	-
Stage 2	-	-	-	-	320	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver		-	-	-	22	361
Mov Cap-2 Maneuver	-	-	-	-	104	-
Stage 1	-	-	-	-	154	-
Stage 2	-	-	-	-	320	-
Approach	EB		WB		SB	
HCM Control Delay, s	0.5		0		20.8	
HCM LOS					С	
Minor Lane/Major Mvn	nt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)		452	-	-	-	327
HCM Lane V/C Ratio		0.142	-	-	-	0.306
HCM Control Delay (s	)	14.3	-	-	-	20.8
HCM Lane LOS		В	-	-	-	С
HCM 95th %tile Q(veh	1)	0.5	-	-	-	1.3

Int Delay, s/veh	0.7					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	٦	- 11	_ <b>≜</b> î≽		Y	
Traffic Vol, veh/h	64	1538	1206	12	5	68
Future Vol, veh/h	64	1538	1206	12	5	68
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	100	-	-	-	0	-
Veh in Median Storage	, # -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	94	94	94	94	94	94
Heavy Vehicles, %	3	3	3	3	3	3
Mvmt Flow	68	1636	1283	13	5	72

Major/Minor	Major1	Ν	/lajor2	-	Minor2	
Conflicting Flow All	1296	0			2244	648
Stage 1	-	-	-	-	1290	-
Stage 2	-	-	-	-	954	-
Critical Hdwy	4.16	-	-	-	6.86	6.96
Critical Hdwy Stg 1	-	-	-	-	5.86	-
Critical Hdwy Stg 2	-	-	-	-	5.86	-
Follow-up Hdwy	2.23	-	-	-	3.53	3.33
Pot Cap-1 Maneuver	525	-	-	-	35	411
Stage 1	-	-	-	-	220	-
Stage 2	-	-	-	-	332	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	r 525	-	-	-	30	411
Mov Cap-2 Maneuver	r -	-	-	-	123	-
Stage 1	-	-	-	-	191	-
Stage 2	-	-	-	-	332	-
Approach	EB		WB		SB	
HCM Control Delay, s	s 0.5		0		18	
HCM LOS					С	
Minor Lane/Major Mv	mt	EBL	EBT	WBT	WBR S	SBLn1
Capacity (veh/h)		525	-	-	-	354
HCM Lane V/C Ratio		0.13	-	-	-	0.219
HCM Control Delay (s	s)	12.9	-	-	-	18
HCM Lane LOS	,	В	-	-	-	С
HCM 95th %tile Q(ve	h)	0.4	_	_	-	0.8

Int Delay, s/veh	1.1					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	٦	<b>^</b>	- <b>†</b> 14		۰¥	
Traffic Vol, veh/h	43	941	1148	13	9	96
Future Vol, veh/h	43	941	1148	13	9	96
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	100	-	-	-	0	-
Veh in Median Storage,	# -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	93	93	93	93	93	93
Heavy Vehicles, %	3	3	3	3	3	3
Mvmt Flow	46	1012	1234	14	10	103

Major/Minor	Major1	Ν	/lajor2		Minor2	
Conflicting Flow All	1248	0	-		1839	624
Stage 1	-	-	-	-	1241	-
Stage 2	-	-	-	-	598	-
Critical Hdwy	4.16	-	-	-	6.86	6.96
Critical Hdwy Stg 1	-	-	-	-	5.86	-
Critical Hdwy Stg 2	-	-	-	-	5.86	-
Follow-up Hdwy	2.23	-	-	-	3.53	3.33
Pot Cap-1 Maneuver	548	-	-	-	66	426
Stage 1	-	-	-	-	234	-
Stage 2	-	-	-	-	509	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver		-	-	-	60	426
Mov Cap-2 Maneuver	r -	-	-	-	159	-
Stage 1	-	-	-	-	214	-
Stage 2	-	-	-	-	509	-
Approach	EB		WB		SB	
HCM Control Delay, s	s 0.5		0		18.8	
HCM LOS					С	
Minor Lane/Major Mv	mt	EBL	EBT	WBT	WBR	SRI n1
Capacity (veh/h)		548	-	-	-	372
HCM Lane V/C Ratio		0.084	-	-		0.304
HCM Control Delay (s	2)	12.2	-	-	-	18.8
HCM Lane LOS	5)	12.2 B		_	_	10.0 C
HCM 95th %tile Q(ve	h)	0.3	_	_	_	1.3
	"'	0.0				1.0

Int Delay, s/veh	0.2					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	۲.	- 11	_ <b>≜</b> î≽		Y	
Traffic Vol, veh/h	9	941	1156	5	9	5
Future Vol, veh/h	9	941	1156	5	9	5
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	25	-	-	-	0	-
Veh in Median Storage	, # -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	93	93	93	93	93	93
Heavy Vehicles, %	3	3	3	3	3	3
Mvmt Flow	10	1012	1243	5	10	5

Major/Minor	Major1	Ν	/lajor2	1	Minor2	
Conflicting Flow All	1248	0	-	0	1772	624
Stage 1	-	-	-	-	1246	-
Stage 2	-	-	-	-	526	-
Critical Hdwy	4.16	-	-	-	6.86	6.96
Critical Hdwy Stg 1	-	-	-	-	5.86	-
Critical Hdwy Stg 2	-	-	-	-	5.86	-
Follow-up Hdwy	2.23	-	-	-	3.53	3.33
Pot Cap-1 Maneuver	548	-	-	-	73	426
Stage 1	-	-	-	-	232	-
Stage 2	-	-	-	-	554	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver		-	-	-	72	426
Mov Cap-2 Maneuver	-	-	-	-	173	-
Stage 1	-	-	-	-	228	-
Stage 2	-	-	-	-	554	-
Approach	EB		WB		SB	
HCM Control Delay, s	0.1		0		22.6	
HCM LOS			-		С	
Miner Long /Maier Mur			грт			1
Minor Lane/Major Mvr	nt	EBL	EBT	WBT	WBR	
Capacity (veh/h)		548	-	-	-	220
HCM Lane V/C Ratio	,	0.018	-	-		0.068
HCM Control Delay (s	5)	11.7	-	-	-	22.6
HCM Lane LOS	. \	B	-	-	-	С
HCM 95th %tile Q(veh	1)	0.1	-	-	-	0.2

Interocotion						
Int Delay, s/veh	0.9					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	٦	- 11	_ <b>≜</b> î≽		Y	
Traffic Vol, veh/h	46	1366	1176	17	11	85
Future Vol, veh/h	46	1366	1176	17	11	85
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	100	-	-	-	0	-
Veh in Median Storage	, # -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	3	3	3	3	3	3
Mvmt Flow	50	1485	1278	18	12	92

Major/Minor	Major1	Ν	/lajor2		Minor2	
Conflicting Flow All	1296	0	-		2130	648
Stage 1	-	-	-	-	1287	-
Stage 2	-	-	-	-	843	-
Critical Hdwy	4.16	-	-	-	6.86	6.96
Critical Hdwy Stg 1	-	-	-	-	5.86	-
Critical Hdwy Stg 2	-	-	-	-	5.86	-
Follow-up Hdwy	2.23	-	-	-	3.53	3.33
Pot Cap-1 Maneuver	525	-	-	-	42	411
Stage 1	-	-	-	-	221	-
Stage 2	-	-	-	-	380	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver		-	-	-	38	411
Mov Cap-2 Maneuver	r -	-	-	-	135	-
Stage 1	-	-	-	-	200	-
Stage 2	-	-	-	-	380	-
Approach	EB		WB		SB	
HCM Control Delay, s			0		20.7	
HCM LOS			Ū		C	
					•	
		EDI	FDT			
Minor Lane/Major Mv	mt	EBL	EBT	WBT	WBR S	
Capacity (veh/h)		525	-	-	-	333
HCM Lane V/C Ratio		0.095	-	-	-	0.313
HCM Control Delay (s	S)	12.6	-	-	-	20.7
HCM Lane LOS		В	-	-	-	С

1.3

HCM 95th %tile Q(veh)

0.3

Int Delay, s/veh 0.5 Movement EBL EBT WBT WBR SBL SBR **†₽** 1173 ¥ Lane Configurations ٦ **↑**↑ 1363 21 Traffic Vol, veh/h 14 5 21 Future Vol, veh/h 14 1363 1173 5 21 21 Conflicting Peds, #/hr 0 0 0 0 0 0 Sign Control Stop Free Free Free Free Stop RT Channelized -None -None -None Storage Length 25 0 -\_ --Veh in Median Storage, # -0 0 -0 -Grade, % 0 0 0 ---Peak Hour Factor 91 91 91 91 91 91 Heavy Vehicles, % 3 3 3 3 3 3 Mvmt Flow 15 1498 1289 5 23 23

Major/Minor	Major1	Ν	/lajor2	1	Minor2	
Conflicting Flow All	1294	0			2071	647
Stage 1	-	-	-	-	1292	-
Stage 2	-	-	-	-	779	-
Critical Hdwy	4.16	-	-	-	6.86	6.96
Critical Hdwy Stg 1	-	-	-	-	5.86	-
Critical Hdwy Stg 2	-	-	-	-	5.86	-
Follow-up Hdwy	2.23	-	-	-	3.53	3.33
Pot Cap-1 Maneuver	526	-	-	-	46	411
Stage 1	-	-	-	-	220	-
Stage 2	-	-	-	-	410	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver		-	-	-	45	411
Mov Cap-2 Maneuver	· -	-	-	-	147	-
Stage 1	-	-	-	-	214	-
Stage 2	-	-	-	-	410	-
Approach	EB		WB		SB	
HCM Control Delay, s	s 0.1		0		26	
HCM LOS					D	
Minor Lane/Major Mvi	mt	EBL	EBT	WBT	WBR S	SRI n1
Capacity (veh/h)		526		101	-	217
HCM Lane V/C Ratio		0.029	-	-		0.213
HCM Control Delay (s	2)	12.1		-	-	26
HCM Lane LOS	<b>)</b>	12.1 B	_	_	-	D
HCM 95th %tile Q(vel	h)	0.1	_	-	-	0.8
	,	5.1				0.0

#### Intersection Int Delay, s/veh 0.9 EBL EBT WBT WBR SBL SBR Movement **†₁** 1389 Lane Configurations ٦ **↑**↑ ¥ 4 Traffic Vol, veh/h 61 1641 24 91 Future Vol, veh/h 61 1641 1389 24 4 91 Conflicting Peds, #/hr 0 0 0 0 0 0 Sign Control Stop Stop Free Free Free Free RT Channelized -None -None -None Storage Length 100 0 -\_ --Veh in Median Storage, # -0 0 -0 -Grade, % 0 0 0 ---Peak Hour Factor 95 95 95 95 95 95 Heavy Vehicles, % 3 3 3 3 3 3 Mvmt Flow 64 1727 1462 25 4 96

Major/Minor	Major1	Ν	/lajor2		Minor2	
Conflicting Flow All	1487	0	-	0	2467	744
Stage 1	-	-	-	-	1475	-
Stage 2	-	-	-	-	992	-
Critical Hdwy	4.16	-	-	-	6.86	6.96
Critical Hdwy Stg 1	-	-	-	-	5.86	-
Critical Hdwy Stg 2	-	-	-	-	5.86	-
Follow-up Hdwy	2.23	-	-	-	3.53	3.33
Pot Cap-1 Maneuver	443	-	-	-	25	355
Stage 1	-	-	-	-	175	-
Stage 2	-	-	-	-	317	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuve		-	-	-	21	355
Mov Cap-2 Maneuve	r -	-	-	-	101	-
Stage 1	-	-	-	-	150	-
Stage 2	-	-	-	-	317	-
Approach	EB		WB		SB	
HCM Control Delay,			0		21.2	
HCM LOS	3 0.5		U		21.2 C	
					U	
Minor Lane/Major Mv	/mt	EBL	EBT	WBT	WBR S	SBLn1
Capacity (veh/h)		443	-	-	-	321
HCM Lane V/C Ratio	1	0.145	-	-	-	0.312
HCM Control Delay (	s)	14.5	-	-	-	21.2
HCM Lane LOS		В	-	-	-	С
HCM 95th %tile Q(ve	eh)	0.5	-	-	-	1.3

Int Delay, s/veh	0.4					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		- 11	_ <b>≜</b> î≽		- ¥	
Traffic Vol, veh/h	12	1634	1393	4	18	21
Future Vol, veh/h	12	1634	1393	4	18	21
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	25	-	-	-	0	-
Veh in Median Storage	e, # -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	3	3	3	3	3	3
Mvmt Flow	13	1720	1466	4	19	22

Major/Minor	Major1	Ν	/lajor2	1	Minor2	
Conflicting Flow All	1470	0	-	0	2354	735
Stage 1	-	-	-	-	1468	-
Stage 2	-	-	-	-	886	-
Critical Hdwy	4.16	-	-	-	6.86	6.96
Critical Hdwy Stg 1	-	-	-	-	5.86	-
Critical Hdwy Stg 2	-	-	-	-	5.86	-
Follow-up Hdwy	2.23	-	-	-	3.53	3.33
Pot Cap-1 Maneuver	450	-	-	-	29	360
Stage 1	-	-	-	-	176	-
Stage 2	-	-	-	-	361	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver		-	-	-	28	360
Mov Cap-2 Maneuver	r -	-	-	-	117	-
Stage 1	-	-	-	-	171	-
Stage 2	-	-	-	-	361	-
Approach	EB		WB		SB	
HCM Control Delay, s	s 0.1		0		30.1	
HCM LOS					D	
Minor Lane/Major Mv	mt	EBL	EBT	WBT	WBR	2DI n1
	m		EDI	VVDI		
Capacity (veh/h)		450	-	-	-	184
HCM Lane V/C Ratio		0.028	-	-		0.223
HCM Control Delay ( HCM Lane LOS	5)	13.2 B	-	-	-	30.1
	b)	о.1	-	-	-	D
HCM 95th %tile Q(ve	n)	0.1	-	-	-	0.8

Int Delay, s/veh	0.7					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	٦	<b>^</b>	- <b>†</b> î»		Y	
Traffic Vol, veh/h	64	1565	1233	12	5	68
Future Vol, veh/h	64	1565	1233	12	5	68
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	100	-	-	-	0	-
Veh in Median Storage	, # -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	94	94	94	94	94	94
Heavy Vehicles, %	3	3	3	3	3	3
Mvmt Flow	68	1665	1312	13	5	72

Major/Minor	Major1	Ν	/lajor2	Minor2		
Conflicting Flow All	1325	0	-	0	2288	663
Stage 1	-	-	-	-	1319	-
Stage 2	-	-	-	-	969	-
Critical Hdwy	4.16	-	-	-	6.86	6.96
Critical Hdwy Stg 1	-	-	-	-	5.86	-
Critical Hdwy Stg 2	-	-	-	-	5.86	-
Follow-up Hdwy	2.23	-	-	-	3.53	3.33
Pot Cap-1 Maneuver	512	-	-	-	33	401
Stage 1	-	-	-	-	212	-
Stage 2	-	-	-	-	326	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver		-	-	-	29	401
Mov Cap-2 Maneuver	• -	-	-	-	119	-
Stage 1	-	-	-	-	184	-
Stage 2	-	-	-	-	326	-
Approach	EB		WB		SB	
HCM Control Delay, s	0.5		0		18.4	
HCM LOS					С	
Minor Lane/Major Mvi	mt	EBL	EBT	WBT	WBR S	RIn1
	IIIL					
Capacity (veh/h) HCM Lane V/C Ratio		512 0.133	-	-	-	345 0.225
HCM Control Delay (s	•)	13.1	-	-	-	18.4
HCM Lane LOS	)	B	-	-	-	10.4 C
HCM 95th %tile Q(vel	h)	0.5	-	-	-	0.8
	1)	0.5		-	-	0.0

Int Delay, s/veh 0.7 EBL EBT WBT WBR SBL SBR Movement **†₽** 1218 Y Lane Configurations ٦ **↑**↑ 27 Traffic Vol, veh/h 21 1548 8 27 Future Vol, veh/h 21 1548 1218 8 27 27 Conflicting Peds, #/hr 0 0 0 0 0 0 Sign Control Free Free Free Free Stop Stop RT Channelized -None -None -None Storage Length 25 0 -\_ --Veh in Median Storage, # -0 0 -0 -Grade, % 0 0 0 ---Peak Hour Factor 93 93 93 93 93 93 Heavy Vehicles, % 3 3 3 3 3 3 Mvmt Flow 23 1665 1310 9 29 29

Major/Minor	Major1	Ν	/lajor2	I	Vinor2	
Conflicting Flow All	1319	0	-	0	2194	660
Stage 1	-	-	-	-		-
Stage 2	-	-	-	-	879	-
Critical Hdwy	4.16	-	-	-	6.86	6.96
Critical Hdwy Stg 1	-	-	-	-	5.86	-
Critical Hdwy Stg 2	-	-	-	-	5.86	-
Follow-up Hdwy	2.23	-	-	-	3.53	3.33
Pot Cap-1 Maneuver	515	-	-	-	38	403
Stage 1	-	-	-	-	213	-
Stage 2	-	-	-	-	364	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver		-	-	-	36	403
Mov Cap-2 Maneuver	r -	-	-	-	134	-
Stage 1	-	-	-	-	203	-
Stage 2	-	-	-	-	364	-
Approach	EB		WB		SB	
HCM Control Delay, s	s 0.2		0		30	
HCM LOS					D	
Minor Lane/Major Mv	mt	EBL	EBT	WBT	WBR S	SBLn1
Capacity (veh/h)		515	-	-	-	201
HCM Lane V/C Ratio		0.044	-	-	-	0.289
HCM Control Delay (s		12.3	-	-	-	30
HCM Lane LOS	,	В	-	-	-	D
HCM 95th %tile Q(vel	h)	0.1	-	-	-	1.1